

20B10083

MASTER PLAN QUAIL

Pasco County Florida
REVIEWED FOR CODE COMPLIANCE
Approval of these documents constitutes
authority to proceed with the work but does not
grant authority to violate, amend, alter, or
set aside any of the technical codes.
[Signature]
11/17/2020

Ultimate Design Wind Speed 141 mph
Risk Category I (II) IV
Florida Building Code 2017 Edition (2017)

CONTRACTOR COPY

GENERAL NOTES:

THE FOLLOWING TECHNICAL CODES SHALL APPLY:
2017 FLORIDA BUILDING CODE,
PLUMBING, MECHANICAL, FUEL GAS,
ENERGY EFFICIENCY, ACCESSIBILITY,
AND NATIONAL ELECTRICAL CODES
NEC 2014

- TANK TYPE WATER CLOSET VOLUME
1.8 GALLONS
- WALL MOUNT WATER CLOSET VOLUME
3.5 GALLONS
- WATER - FLOW RATE.
PUBLIC FACILITIES 0.5 G.P.M.
PRIVATE FACILITIES 2.2 G.P.M.
SHOWER HEADS 2.5 G.P.M.

VTR LOCATIONS ARE APPROXIMATE
AND MAY CHANGE DUE TO JOBSITE
CONDITIONS

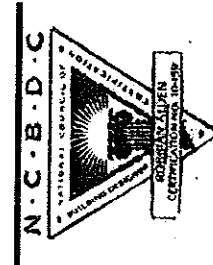
THE FOLLOWING SHALL COMPLY
WITH THE 2017 FBC.

- PORCHES AND BALCONIES
 - HANDRAILS
 - GUARDRAILS
 - STAIRS
 - CHIMNEY & FIREPLACE
 - EGRESS WINDOWS
- ALL OPENINGS SHALL COMPLY WITH
2017 FBC WIND LOADS AS STATED
BELOW. ATTACHMENTS OF WINDOWS,
DOORS, SLIDING GLASS DOORS
AND O.H. GARAGE DOORS ARE DELEGATED
THE MANUFACTURER OF THESE ITEMS. THE
MANUFACTURER OF THESE ITEMS
SHALL SUBMIT ATTACHMENTS TO ENGINEER
OF RECORD FOR REVIEW PRIOR TO INSTALLATION.
SEE ATTACHED SPECIFICATION SHEETS FOR
MANUFACTURERS DESIGN CRITERIA AND
INSTALLATION METHODS FOR WINDOWS,
DOORS, SLIDING GLASS DOORS, OVERHEAD
GARAGE DOORS, AND ROOFING.
 - ALL DOORS INTERIOR & EXTERIOR ARE
8' 0" UNLESS OTHERWISE NOTED
ALL SHOWER ENCLOSURES TO BE
TEMPERED GLASS
 - ALL WINDOWS WITHIN 24" OF DOORS
(INTERIOR & EXTERIOR) AND WITHIN
18" OFF FLR TO BE TEMPERED GLASS.

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SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

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ALLEN ENGINEERING AND CONSTRUCTION SERVICES, INC. (AECS) IS NOT
RESPONSIBLE FOR THE ARCHITECTURAL DESIGN, ITS FEATURES AND
ASSOCIATED DIMENSIONS. THE ARCHITECTURAL INFORMATION IS ACCEPTED
AS BEING ACCURATE AND IS USED BY AECS SOLELY FOR THE PURPOSE OF
DETERMINING STRENGTH, FIRE PROTECTION, AND FLOOD RESISTANCE
CONSTRUCTION REQUIREMENTS.

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NOTICE TO SUBCONTRACTORS :

DUE TO SPACE LIMITATIONS IN THIS 11"X 17" PLAN
FORMAT, AND TO ELIMINATE CLUTTER AND TEXT
READABILITY ISSUES, SOME DETAILS AND NOTATIONS
MAY OR MAY NOT BE LOCATED ON THE SAME SHEETS
OR IN THE SAME LOCATIONS AS PROVIDED FOR BY
OTHER CONTRACTORS OR ARCHITECTS.
IT WOULD BE IN YOUR BEST INTREST TO REVIEW THESE
PLANS AND LOCATE THE APPROPRIATE INFORMATION
REQUIRED TO COMPLETE YOUR SPECIFIC PORTION OF
THE JOB BEFORE BEGINNING CONSTRUCTION.

NOTICE TO BUILDER

IT IS THE INTENT OF THIS DESIGNER THAT
THESE PLANS ARE ACCURATE AND ARE
CLEAR ENOUGH FOR THE LICENSED PROFESSIONAL
TO CONSTRUCT THIS PROJECT.
IN THE EVENT THAT SOMETHING IS UNCLEAR
OR NEEDS CLARIFICATION, STOP AND CALL
THE DESIGNER LISTED IN THIS TITLE PAGE. IT
IS THE RESPONSIBILITY OF THE LICENSED
PROFESSIONAL THAT IS CONSTRUCTING THIS
PROJECT TO FULLY REVIEW THESE DOCUMENTS
BEFORE CONSTRUCTION BEGINS AND ANY AND
ALL CORRECTIONS, IF NEEDED, TO BE MADE
BEFORE ANY WORK IS DONE.

WINDOW INSTALLATION NOTES:

- WINDOWS MUST BE FASTENED INTO STRUCTURAL MEMBERS
PER MFG'S. DETAIL REQUIREMENTS PER DESIGN CRITERIA
NOTED ON THESE DRAWINGS.
- WINDOWS ARE IMPACT RESISTANT TYPE. NO STORM
SHUTTERS OR PANELS ARE REQUIRED.
- ROOF, WALLS AND WINDOW FASTENINGS MUST BE
ENGINEERED AND SPECIFIED FOR CUMULATIVE INTERNAL
PRESSURE AND EXTERNAL NEGATIVE (SUCTION) PRESSURES
WHICH VARIES ACCORDING TO AREAS AS NOTED IN THE DESIGN
CRITERIA AS NOTED ON PAGE S4.

A.E.C.S. 20046 QUAIL MODEL

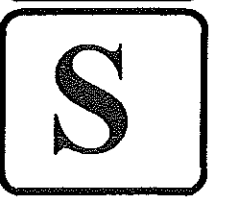
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HEREBY CERTIFY THAT I HAVE
PERFORMED THE ATTACHED DESIGN
TO COMPLY WITH 145 MPH ULTIMATE
WIND LOADS AND IT IS IN COMPLIANCE
WITH SECT. 301 OF THE 2017 FLORIDA
RESIDENTIAL BUILDING CODE
SEALED FOR STRUCTURE ONLY
SIGNED: *[Signature]*
RICHARD LALLEN P.E. #56920

BUILDING 10
RESERVE AT HUNTERS RIDGE
PHASE TWO

PLAN DATE
04-28-20

DEEB FAMILY
HOMES, LTD.
9400 RIVER CROSSING BLD.
NEW PORT RICHEY, FL. 34655
727-376-6831



COVER SHEET

CONTRACTOR COPY

21. FLOORS
- A. MANUFACTURED FLOOR TRUSS FRAMING PLAN CONTAINED HEREIN IS FOR THE SOLE PURPOSE OF ILLUSTRATING THE DESIGN INTENT AND FOR PLANNING TO BE USED BY THE TRUSS COMPANY.
- I. FLOOR JOISTS ARE SIZED BASED ON THE SOUTHERN PINE COUNCIL SPAN TABLES FOR NO. 2 GRADE DIMENSIONAL LUMBER.
 - II. FLOOR JOISTS FOR EXTERIOR DECKS SHALL BE PRESSURE TREATED.
- B. FOR ALL WOOD FLOORS:
- I. THE TRUSS TO WALL CONNECTIONS ARE IDENTIFIED ON THE FLOOR FRAMING PLAN.
 - II. A STRUCTURAL BAND JOIST IS TO BE PROVIDED ON THE EXTERIOR PERIMETER OF ALL BOTTOM BEARING FLOOR TRUSSES AND JOISTS. THE STRUCTURAL BAND JOIST IS TO BE FASTENED TO EACH END OF A FLOOR TRUSS OR JOIST WITH A SIMPSON L50 BRACKET USING SIMPSON SHORT 10d COMMON NAILS.
 - III. FLOOR TRUSSES OR JOISTS BEARING ON WOOD WALLS ARE TO BE SET WITH A MINIMUM OF THREE 10d COMMON NAILS (TOE NAILED) TO THE TOP PLATE OF THE WALL.
 - IV. A MOISTURE BARRIER SHALL BE INSTALLED BETWEEN ANY UNTREATED WOOD TRUSSES OR JOISTS AND CONCRETE OR ANY MASONRY.
 - V. LEDGERS/ NAILERS SHALL BE FASTENED TO WOOD STUDS OR BAND JOISTS (NOT SHEATHING) WITH A MINIMUM 2 3/8" X 5 1/2" LAG BOLTS WITH WASHERS AT EACH STUD INTERSECTION AT 16 INCHES ON CENTER AND SHALL CONSIST OF PRESSURE TREATED LUMBER 2 PLY 1 1/2" THICK BY A HEIGHT SHOWN IN THE PLANS. FOR CONCRETE OR MASONRY WALLS THE FASTENERS SHALL BE 5/8" X 5 1/2" SIMPSON TITEN HEAD CONCRETE BOLTS.
- VI. FLOOR BEAMS
1. BEAMS SUPPORTING FLOOR TRUSSES AND JOISTS ARE TO BE ATTACHED AS SPECIFIED IN THE FLOOR FRAMING PLAN. UNDER NO CIRCUMSTANCES ARE THERE TO BE BUTT JOINTS BETWEEN THE BEARING POINTS OF ANY PLY OF A MULTIPLE BEAM. THE PLYS ARE TO BE CONTINUOUS BETWEEN BEARING POINTS.
 3. MULTIPLE BEAMS CONSISTING OF MANUFACTURED WOOD (I.E. GLULAM , MICROLAM) ARE TO HAVE THE INDIVIDUAL PLYS INTERCONNECTED AS REQUIRED BY THE MANUFACTURERS SPECIFICATIONS.
 4. MULTIPLE BEAMS CONSISTING OF DIMENSIONAL LUMBER ARE TO HAVE INDIVIDUAL PLYS INTERCONNECTED AS FOLLOWS:
 - A. FOR TWO PLY BEAMS- ONE ROW OF 10d GALVANIZED COMMON NAILS AT 6" O.C. ON EACH SIDE OF THE BEAM
 - B. FOR THREE PLY BEAMS- TWO ROWS OF 16d GALVANIZED COMMON NAILS SPACED AT 6" O.C. (TOP AND BOTTOM) THRU EACH SIDE OF BEAM.
 - C. FOR FOUR PLY BEAMS OR LARGER- TWO ROWS OF 1/2" DIAMETER CARRIAGE BOLTS OR ALL THREAD ROD WITH NUTS AND WASHERS SPACED AT 12 INCHES ON CENTER, 2 INCHES FROM THE TOP AND BOTTOM EDGES OF THE BEAM.
- D. FLOOR SHEATHING :
- I. ALL FLOOR SHEATHING IS TO BE 3/4" TONGUE AND GROOVE PLYWOOD RATED FOR FLOOR SHEATHING APPLICATION.
 - II. FLOOR SHEATHING SHALL BE FASTENED TO THE FLOOR TRUSSES /JOISTS WITH 10d RING SHANK NAILS AT 6" ON CENTER WITH CONSTRUCTION GRADE ADHESIVE.
 - III. FLOOR SHEATHING SPECIFIED FOR SEALED EXTERIOR DECKS AND ITS INSTALLATION SHALL BE THE SAME AS THAT FOR INTERIOR APPLICATION EXCEPT PRESSURE TREATED AND THE FASTENERS TO BE GALVANIZED.
- E. EXTERIOR DECK FLOORING:
- I. DECK FLOORING SHALL BE INDIVIDUALLY SPECIFIED ON THE FLOOR FRAMING PLANS AND SHALL BE FASTENED TO THE UNDERLYING PRESSURE TREATED JOISTS WITH 3- 3 INCH DECK SCREWS AE EACH FLOORING JOIST INTERSECTION.

22. WALLS :
- A. MASONRY
- I. CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI.
 - II. WALL CMU SHALL BE 8 INCH X 16 INCH IN SIZE OR 8 INCH X 8 INCH X 8 INCH FOR EDGE FINISHES.
 - III. CMU SHALL BE PLACED IN A RUNNING BOND AND THERE SHALL BE NO VERTICAL BUTT JOINTS EXCEPT AS SHOWN ON THE FLOOR PLAN FOR CONSTRUCTION JOINTS.
 - IV. REINFORCED FILLED CELLS AS SHOWN ON THE PLANS SHALL BE FILLED WITH " FINE" GRADE GROUT , HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI AND 8 TO 11 INCH SLUMP TO ENSURE CONSOLIDATION.
 - V. BOND BEAMS SHALL BE POURED WITH GROUT MONOLITHICALLY WITH THE FILLED WALL CELLS-NO COLD JOINTS.
 - VI. VERTICAL STEEL REINFORCEMENT SHALL BE CONTINUOUS BETWEEN THE MIDDLE AND BOTTOM 1/3 OF THE FOOTING HEIGHT AND END IN THE TOP COURSE OF THE BOND BEAM WITH A STANDARD 10 INCH 90 DEGREE BEND.
 - VII. HORIZONTAL REINFORCING STEEL SHALL BE CONTINUOUS, INCLUDING AROUND CORNERS.
 - VIII. REINFORCING STEEL SPLICES SHALL CONSIST OF WIRE LAPS NO LESS THAN 40 TIMES THE STEEL BAR DIAMETER (I.E. 25 INCHES FOR #5 REBAR, 15 INCHES FOR #3 REBAR, AND 52 INCHES FOR #7 REBAR)
- B. WOOD FRAME WALLS :
- I. WALL STUD SIZES ARE SHOWN IN THE TYPICAL WALL SECTION.
 - II. LOAD BEARING.
 1. WOOD STUDS IN WALLS SHALL BE SPACED 16 INCHES ON CENTER AND FASTENED TO THE TOP AND BOTTOM PLATES PER THE TOP PLATE SPLICE DETAIL. ALL LOAD BEARING STUDS TO BE SOUTHERN YELLOW PINE #2 GRADE OR BETTER.
 2. LOAD BEARING WALLS SHALL HAVE A SINGLE BOTTOM PLATE (PRESSURE TREATED) IN CONTACT WITH MASONRY OR CONCRETE. SEE THE TOP PLATE SPICE DETAIL FOR TOP PLATE NAILING AND SPLICING REQUIREMENTS.
 3. THE WOOD STUDS SHALL HAVE A SIMPSON SP2 AT THE TOP PLATE AND A PROPERLY SIZED SPH FOR THE BOTTOM PLATE (I.E. 4" STUD WALL = SPH4, 6" STUD WALL = SPH6)
 4. 3 STUD PACK SHALL BE INSTALLED DIRECTLY BENEATH BEARING POINTS OF ALL GIRDERS AND BEAMS HAVING A GRAVITY LOAD OF UP TO 3,000 LBS.
 5. STEEL TUBE COLUMNS SHALL BE INSTALLED IN THE WALL DIRECTLY BENEATH GIRDERS AND BEAMS HAVING GRAVITY LOADS GREATER THAN 3000 LBS.
 6. BASE PLATES SHALL BE FASTENED TO MONOLITHIC FOOTINGS WITH 5/8" X 8 INCH ANCHOR BOLTS OR SIMPSON TITEN HD. CONCRETE BOLTS OF THE SAME SIZE AT 24 INCHES ON CENTER. ALL CONNECTIONS SHALL BE MADE WITH 3 INCH SQUARE BY 1/8 INCH THICK WASHERS
 7. BASE PLATES BEARING ON WOOD SHALL BE FASTENED WITH 16d COMMON NAILS AT 8" O.C. THROUGH ANY FLOOR SHEATHING AND TO UNDERLYING LUMBER (NOT SHEATHING ONLY) AND USE BLOCKING AS NEEDED TO MAINTAIN NAILING SPACING REQUIREMENTS.
 8. FOR EXTERIOR LOAD BEARING WALLS, EACH STUD ABOVE THE BASE PLATE SHALL BE FASTENED TO THE UNDERLYING BAND JOIST OR BEAM WITH A SIMPSON LSTA18 STRAP. FOR THIS SITUATION THE SIMPSON SPH BRACKET TO THE BASE PLAN MAY BE OMITTED.
 9. FOR INTERIOR LOAD BEARING WALLS, 1/2 INCH ALL THREAD ROD SHALL BE INSTALLED AT 32" O.C. FROM THE BASE PLATE THROUGH THE SHEATHING AND TOP PLATE OF UNDERLYING SUPPORTING WALL. ALL CONNECTIONS SHALL INCLUDE A STANDARD 3 INCH SQUARE WASHER.
 10. HEADER BEAMS SHALL BE SIZED ACCORDING TO THE ENCLOSED HEADER SCHEDULE AND FASTENED WITH A MINIMUM OF TWO SIMPSON LSTA36 STRAPS OVER EACH END TO THE JACK STUDS BELOW. IN ADDITION, THE HEADER BEAMS SHALL BE FASTENED WITH A MINIMUM OF 3-10d COMMON NAILS (TOE NAILED ON EACH FACE SIDE AT EACH END TO THE ABUTTING FULL LENGTH STUDS.
- III. NON LOAD BEARING WALLS:
1. WOOD STUDS IN WALLS SHALL BE SPACED AT 16 INCHES ON CENTER AND FASTENED TO THE TOP AND BOTTOM PLATES WITH A MINIMUM OF THREE 10d COMMON NAILS. NAILS INSTALLED IN PRESSURE TREATED WOOD SHALL BE GALVANIZED.
 2. INCIDENTAL, NON STRUCTURAL FRAMING ITEMS SUCH AS KNEE WALLS, DROP CEILINGS , BUILT IN SHELVING , NICHES, ETC. MAY BE CONSTRUCTED WITH 2 X 4'S AT 24" O.C. AT THE DISCRETION OT THE BUILDER.

2. NON LOAD BEARING WALLS SHALL HAVE A SINGLE BOTTOM PLATE (PRESSURE TREATED AGAINST MASONRY AND CONCRETE) AND A SINGLE TOP PLATE.
 3. BASE PLATES SHALL BE FASTENED TO CONCRETE SLABS WITH 1/4 INCH BY 3 1/2 INCH TAPCON SCREWS AT 12 " ON CENTER.
 4. BASE PLATES ON WOOD SHALL BE FASTENED WITH 16d COMMON NAILS AT 8" ON CENTER.
- C. SHEATHING
- I. PLYWOOD SHEATHING.
1. EXTERIOR WALL SHEATHING COVERED BY AN ARCHITECTURAL FINISH SHALL BE MINIMUM 7/16 INCH THICK (NOMINAL) 4 PLY PLYWOOD MANUFACTURED WITH EXTERIOR GLUE.
 2. THE LONG SIDE OF THE SHEATHING SHALL BE INSTALLED PERPENDICULAR TO THE WALL STUDS.
 3. FASTEN TO STUDS AND BLOCKING WITH 8d RING SHANK NAILS AT 4 INCHES ON CENTER ALL LOCATIONS.
 4. IN ADDITION TO THE REGULAR FASTENING, A SECOND ROW SHALL BE INSTALLED AT THE DOUBLE TOP PLATE AND TO THE LOWEST HORIZONTAL WOOD MEMBER ON AN EXTERIOR WALL. (I.E. SILL PLATE , BAND JOIST)
 5. FOR PLYWOOD SHEATHING COVERED WITH A CEMENTITIOUS FINISH ALL BUTT JOINTS NOT ON WALL STUDS SHALL BE BLOCKED WITH 2 X BLOCKING , TOE NAILED AT EACH END TO THE WALL STUDS WITH 3-8d COMMON NAILS.
- II. PARTICLE BOARD
1. PARTICLE BOARD IS NOT TO BE USED WITHOUT THE EXPRESS, WRITTEN CONSENT OF THE STRUCTURAL ENGINEER AND THE PROPERTY OWNER.
- III. ARCHITECTURAL FINISHES
1. ARCHITECTURAL WALL FINISHES , SUCH AS STUCCO, CEMENTITIOUS COATING , SIDING OR PAINT ARE MENTIONED HERE ONLY FOR THE PURPOSE OF UNDERSTANDING THAT THEIR INSTALLATION AND ASSOCIATED DETAILS ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
23. COLUMNS
- A. CONCRETE / MASONRY COLUMNS
1. MASONRY COLUMNS SHALL BE CONSTRUCTED OF PILASTER CONCRETE BLOCK OR FORMED AND POURED. WALL BLOCK SHALL NOT BE USED FOR MASONRY COLUMNS.
 - II. REINFORCING STEEL SHALL BE GRADE 60 AND HELD IN PLACE BY STRUPS SPACED AT 12 INCHES ON CENTER VERTICALLY.
 - III. PILASTER BLOCK COLUMNS SHALL BE FILLED WITH A FINE GROUT HAVING A MINIMUM OF COMPRESSIVE STRENGTH OF 3,000 PSI
 - IV. FORMED AND POURED COLUMNS SHALL CONSIST OF A MINIMUM OF 3,000 PSI CONCRETE, OR IN AREAS OF HIGH CHLORIDES, SUCH AS NEAR THE COAST OR BODIES OF SALT WATER , THE MINIMUM SHALL BE 5,000 PSI
 - V. ALL MASONRY COLUMNS SHALL BEGIN AT THE FOUNDATION OR AT A MONOLITHIC FOOTING. IN NO CASE SHALL THERE BE A BREAK OR A COLD JOINT IN THE GROUT OF A COLUMN EXCEPT AT 1 FOOT FROM THE TOP IN PREPARATION FOR INSTALLATION OF A CONCRETE LINTEL.
- VI. METAL CONNECTORS AT THE TOP OF THE COLUMN FOR HOLDING WOOD BEAMS OR GIRDERS SHALL BE INSTALLED WITH THE MINIMUM EMBEDMENT OF THE ASSOCIATED FASTENERS FOR THE CONNECTOR AS SHOWN ON THE PLANS.
- B. WOOD COLUMNS :
- I. ALL LOAD BEARING WOOD COLUMNS SHALL BE A MINIMUM OF #2 GRADE PRESSURE TREATED WOOD.
 - II. DIMENSIONAL WOOD COLUMNS OF 4 INCHES BY 4 INCHES IN CROSS SECTION SHALL ONLY BE USED FOR SUPPORTING OPEN WOOD DECKS WHERE THE FLOOR HEIGHT ABOVE THE FLOOR BELOW IS 8 FEET OR LESS. ALL OTHER DIMENSIONAL WOOD COLUMNS SHALL HAVE A MINIMUM OF 6 INCHES BY 6 INCHES.
 - III. METAL CONNECTORS AT THE BASE AND THE TOP OF WOOD COLUMNS SHALL BE OF THE TYPE THAT RESISTS LATERAL LOADS AS WELL AS UPLIFT AND GRAVITY LOADS. IN NO CASE SHALL FLAT STRAPS BE USED UNLESS SPECIFICALLY SHOWN IN THE PLANS OR CROSS SECTION DETAILS.

STRUCTURAL ENGINEER NOTES



DEEB FAMILY
HOMES, LTD.
9400 RIVER CROSSING BLD.
NEW PORT RICHEY, FL. 34655
727-376-6831

PLAN DATE

04-28-20

BUILDING 10

RESERVE AT HUNTERS RIDGE
PHASE TWO

A.E.C.S. 20046

QUAIL MODEL

HEREBY CERTIFY THAT I HAVE PERFORMED THE ATTACHED DESIGN TO COMPLY WITH 145 MPH ULTIMATE WIND LOADS AND IT IS IN COMPLIANCE WITH SECT. 301 OF THE 2017 FLORIDA RESIDENTIAL BUILDING CODE
SEALED FOR STRUCTURE ONLY
SIGNED: RICHARD E. ALLEN P.E. #56920

ALLEN ENGINEERING & CONSTRUCTION SERVICES
RICH ALLEN PROFESSIONAL ENGINEER
P.E. # 56920 C.A. # 9542
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NEW PORT RICHEY, FL. 34656
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richallenpe@gmail.com

C. COMPOSITE COLUMNS

- I. A COMPOSITE COLUMN HERE IS DEFINED AS A HOLLOW COLUMN CONSISTING OF ANY MATERIAL SPECIFICALLY DESIGNED BY ITS MANUFACTURER TO BE LOAD BEARING. ANY OTHER TYPE OF HOLLOW COLUMN IS CONSIDERED AN ARCHITECTURAL FINISH INTENDED TO FIT OVER A STRUCTURAL COLUMN AND ITS USE AND DETAILS OF INSTALLATION ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
- II. LOAD BEARING COMPOSITE COLUMNS ARE A MANUFACTURED PRODUCT SUBJECT TO THE DESIGN AND LOAD BEARING CAPACITY AS DETERMINED BY THE MANUFACTURER. A SHOP DRAWING OR A LETTER FOR THE INSTALLATION OF THE COLUMN SHALL BE PROVIDED BY THE STRUCTURAL ENGINEER TO SUPPLEMENT THE CONSTRUCTION PLANS AFTER THE SPECIFIC COLUMN AND MANUFACTURER HAVE BEEN IDENTIFIED.
- III. IN ALL CASES, THE COLUMN MANUFACTURERS INFORMATION SHALL BE PROVIDED TO THE STRUCTURAL ENGINEER BY THE CONTRACTING CLIENT OR HIS AGENT FOR REVIEW PRIOR TO ITS ACCEPTANCE FOR THE STRUCTURAL DESIGN. THE INFORMATION SHALL INCLUDE THE LATERAL AS WELL AS UPLIFT AND GRAVITY LOAD BEARING CAPACITIES.
- D. STEEL TUBE COLUMNS:
 - I. LOAD BEARING STEEL TUBE COLUMNS SHALL HAVE A MINIMUM WALL THICKNESS OF 1/4 INCH AND BE MADE OF STEEL WITH A DESIGN YIELD STRENGTH OF 46 PSI UNLESS OTHERWISE SHOWN IN THE STRUCTURAL DESIGN
 - II. THE SPECIFIC CONNECTION SCHEME SHALL BE SHOWN IN THE STRUCTURAL DESIGN WHERE THE STEEL TUBE COLUMN IS TO BE INSTALLED.

E. ALUMINUM COLUMNS:

- I. LOAD BEARING ALUMINUM COLUMNS SHALL HAVE A MINIMUM WALL THICKNESS OF 1/4 INCH.
- II. ALL FASTENERS AND CONNECTORS FOR ALUMINUM COLUMNS SHALL BE STAINLESS STEEL OR MONEL TO AVOID CORROSION DUE TO DISSIMILAR METALS BEING IN CONTACT.
- III. THE SPECIFIC CONNECTION SCHEME SHALL BE SHOWN IN THE STRUCTURAL DESIGN WHERE THE ALUMINUM COLUMN IS TO BE INSTALLED.

24. ROOF

A. MANUFACTURED WOOD TRUSSES

- I. THE MANUFACTURED ROOF TRUSS FRAMING PLAN CONTAINED HEREIN IS FOR THE SOLE PURPOSE OF ILLUSTRATING THE DESIGN INTENT AND FOR PLANNING TO BE USED BY THE TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER OF THE TRUSS MANUFACTURER IN DEVELOPING THE ACTUAL SYSTEM DESIGN. IT IS NOT INTENDED TO BE USED FOR ANY OTHER PURPOSE AS IT IS SUBJECT TO ENGINEERING AND MAY BE DIFFERENT FROM THE FINAL DESIGN.
- II. MANUFACTURED ROOF TRUSSES SHALL BE DESIGNED BY A LICENSED TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER ACTING AS A DELEGATED ENGINEER AND WORKING THROUGH A TRUSS MANUFACTURER FOR THIS PURPOSE. THE SELECTION OF THE TRUSS MANUFACTURER IS HEREBY SUBORDINATED TO THE BUILDING CONTRACTOR.
- III. THE TRUSS PLAN "SIGNED AND SEALED" BY THE DELEGATED ENGINEER SHALL BE PROVIDED TO AND PRIOR TO CONSTRUCTION OF THE UNDERLYING STRUCTURE AS THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO MAKE STRUCTURAL CHANGES BASED ON THE FINAL FLOOR TRUSS SYSTEM.
- VI. THE TRUSS MANUFACTURER SHALL PROVIDE ALL LATERAL BRACING REQUIREMENTS TO THE BUILDING CONTRACTOR. IF NOT, THE BUILDING CONTRACTOR IS TO NOTIFY THE STRUCTURAL ENGINEER FOR GUIDANCE.
- V. IN ADDITION TO THE METAL CONNECTORS SHOWN IN THE TRUSS LAYOUT OF THE ORIGINAL PLANS, EACH TRUSS IS TO BE SET ON WOOD FRAME BEARING WALLS OR SILL PLATES WITH 10d COMMON NAILS (TOE-NAILED)
- VI. A MOISTURE BARRIER IS TO BE INSTALLED BETWEEN UNTREATED WOOD AND CONCRETE / MASONRY

23.2 CONVENTIONAL FRAME

- I. IN ADDITION TO THE METAL CONNECTORS SHOWN IN THE TRUSS LAYOUT OF THE ORIGINAL PLANS, EACH RAFTER IS TO BE SET ON WOOD FRAME BEARING WALLS OR SILL PLATES WITH 3- 10d COMMON NAILS (TOE-NAILED)
- II. ANY WOOD COMING IN CONTACT WITH MASONRY OR CONCRETE IS TO BE PRESSURE TREATED OR A MOISTURE BARRIER IS TO BE INSTALLED BETWEEN UNTREATED WOOD AND CONCRETE OR MASONRY.

- III. COLLAR TIES ARE TO BE INSTALLED BETWEEN RAFTERS AT 2/3 OF THE RIDGE HEIGHT FROM WHERE THE RAFTERS BEAR ON WALLS. THE COLLAR TIES ARE TO BE FASTENED WITH A MINIMUM OF 4-10d 16 COMMON NAILS (CLINCHED) AT EACH LAP JOINT. EACH RAFTER IS TO BE ATTACHED TO THE RIDGE BEAM WITH A LIGHT ANGLE HANGER AS SHOWN IN THE FRAMING PLAN. IN ADDITION, A FLAT METAL STRAP SHALL BE INSTALLED ACROSS THE RIDGE BEAM TO TWO OPPOSING RAFTER. TO BE REVIEWED BY THE STRUCTURAL ENGINEER FOR COMPLYING WITH THE DESIGN INTENT OF THE ORIGINAL PLAN AND FOR ANY CHANGES TO THE "TRUSS TO THE UNDERLYING STRUCTURE" CONNECTIONS.

- IV. AS PART OF THE REVIEW, THE STRUCTURAL ENGINEER WILL DETERMINE WHETHER THE TRUSS TO WALL / BEAM METAL CONNECTORS SHOWN IN THE ORIGINAL PLANS ARE ACCEPTABLE OR WHETHER THEY NEED TO BE CHANGED OR SUPPLEMENTED TO ACCOMMODATE THE LOADS SHOWN IN THE TRUSS COMPONENT SHEETS.

- V. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR VERIFYING THE DIMENSIONAL, ARCHITECTURAL, OR FORM ASPECTS OF THE OF THE TRUSS MANUFACTURERS PLAN WITH THE ORIGINAL PLANS.

- VI. THE MINIMUM LIVE LOADS FOR THE ROOF TRUSS DESIGN IS TO BE ON FBC 2017 SECTION 1607 FOR ROOF TYPE AND ROOFING MATERIAL.

- VII. THE DEAD LOADS ARE LISTED IN ITEM 16 ABOVE.
- VIII. ALL TRUSS TO TRUSS AND TRUSS TO GIRDER CONNECTORS ARE TO BE SPECIFIED BY THE TRUSS MANUFACTURER, INCLUDING CONNECTORS FOR TRUSS TO MANUFACTURED BEAM (I.E. GLUELAM, OR MICROLAM) SPECIFIED BY THE TRUSS MANUFACTURER. A SPECIFIC HANGER MUST BE SELECTED AND IDENTIFIED ON THE SIGNED AND SEALED COMPONENT SHEETS FOR EACH LOCATION, A HANGER IS REQUIRED IN THE TRUSS SYSTEM.

- IX. THE TRUSS PLAN SIGNED AND SEALED BY THE DELEGATED ENGINEER SHALL BE PROVIDED TO AND REVIEWED BY THE STRUCTURAL ENGINEER FOR COMPLYING WITH THE DESIGN INTENT OF THE ORIGINAL PLAN AND FOR ANY CHANGES TO THE "TRUSS TO UNDERLYING STRUCTURE" CONNECTIONS. THIS PLAN MUST BE PROVIDED TO THE STRUCTURAL ENGINEER.

- X. A RIDGE BEAM TERMINATING AT A GABLE END SHALL BE SUPPORTED BY A MINIMUM 3 STUD PACK COLUMN BEARING ON THE UNDERLYING WALL OR BEAM.

- XI. TREATED LUMBER-DOUBLE 1 1/2 INCH BY A HEIGHT SHOWN ON THE PLANS. FOR CONCRETE OR MASONRY WALLS THE FASTENERS SHALL BE 5/8 INCH BY 5 1/2 INCH SIMPSON TITEN HD CONCRETE BOLTS.

- XII. SLEEPERS SHALL BE FASTENED TO UNDERLYING ROOF TRUSSES OR RAFTERS (NOT SHEATHING) WITH A MINIMUM OF 2-3/8 INCH BY 3 1/2 INCH LAG BOLTS AND WASHERS AT EACH TRUSS OR RAFTER INTERSECTION AND NO GREATER THAN 24 INCHES ON CENTER AND SHALL CONSIST OF DIMENSIONAL LUMBER 1 1/2 INCH THICK BY A WIDTH SHOWN IN THE PLANS.

- XIII. USE 2 INCH BY 4 INCH BLOCKING ATTACHED BETWEEN UNDERLYING STUDS, TRUSSES OR RAFTERS WITH A MINIMUM OF 3-10d NAILS AT EACH IN ORDER TO SATISFY THE ON CENTER SPACING FOR THE LEDGERS OR SLEEPERS.

- XIV. BEAMS SUPPORTING ROOF TRUSSES OR RAFTERS ARE TO BE ATTACHED AS SPECIFIED IN THE ROOF FRAMING PLANS.

24. UNDER NO CIRCUMSTANCES ARE THERE TO BE BUTT JOINTS BETWEEN THE BEARING POINTS OF ANY PLY OF A MULTIPLE BEAM. THE PLYS ARE TO BE CONTINUOUS BETWEEN BEARING POINTS.

A. LEDGERS/ SLEEPERS

- I. LEDGERS / NAILERS SHALL BE FASTENED TO WOOD STUDS (NOT SHEATHING) WITH A MINIMUM OF 2- 3/8 INCH BY 5 1/2 INCH LAG BOLTS WITH WASHERS AT EACH STUD INTERSECTION AND NO GREATER THAN 16 INCHES ON CENTER AND SHALL CONSIST ON PRESSURE TREATED WOOD.

- II. MULTIPLE BEAMS CONSISTING OF MANUFACTURED WOOD (I.E. GLUELAM, MICROLAM) ARE TO HAVE THE INDIVIDUAL PLYS INTERCONNECTED AS REQUIRED BY THE MANUFACTURERS SPECIFICATIONS.

- III. MULTIPLE BEAMS CONSISTING OF DIMENSIONAL LUMBER ARE TO HAVE THE INDIVIDUAL PLYS INTERCONNECTED AS FOLLOWS:

- I. FOR TWO PLY BEAMS - ONE ROW OF 10d GALVANIZED COMMON NAILS AT 6 INCHES ON CENTER ON EACH SIDE OF BEAM.
- II. FOR THREE PLY BEAMS- TWO ROWS OF 16d GALVANIZED COMMON NAILS AT 6" ON CENTER (TOP AND BOTTOM) THRU EACH SIDE OF THE BEAM.
- III. FOR FOUR PLY BEAMS AND LARGER- TWO ROWS OF 1/2 INCH DIAMETER CARRIAGE BOLTS OR ALL THREAD RODS WITH NUTS AND WASHERS SPACED AT 12" ON CENTER 2 INCHES FROM THE TOP AND BOTTOM EDGES OF THE BEAM.

- IV. FASTENING SHALL BE 8d RING SHANK NAILS AT 4 INCHES ON CENTER AT BOUNDARY AND EDGES AND 6 INCHES ON CENTER IN THE FIELD WITH A SETBACK OF 5'-0" FROM ALL EDGES.

- V. METAL "H" CLIPS OR SOLID WOOD BLOCKING SHALL BE USED AT ALL UNSUPPORTED BUTT JOINTS BETWEEN TRUSSES OR RAFTERS.

25. PRECAST CONCRETE LINTELS

- A. PRECAST AND PRESTRESSED CONCRETE LINTELS SHALL BE MANUFACTURED BY CASTCRETE AND INSTALLED PER MANUFACTURES SPECIFICATIONS AND INSTRUCTIONS.

- B. THE SIZE OF THE LINTELS SHALL BE BASED ON THE SPAN AND LOAD. REFER TO THE ATTACHED SCHEDULE UNLESS OTHERWISE SHOWN IN THE STRUCTURAL DESIGN FOR THE SPECIFIED LINTEL

- C. LINTEL SCHEDULE U.N.O. ON PLANS:
 - I. SPAN UP TO 3'- 8F8-0B
 - II. SPAN UP TO 3' TO < 6' - 8F8-0B
 - III. SPAN 6' TO > 14' - 8F16- 1B/1T

- D. THE MINIMUM SPECIFIED GROUT COMPRESSIVE STRENGTH TO BE USED FOR LINTELS IS 3,000 PSI.

- E. THE REINFORCING STEEL SHALL BE ASTM GRADE 60

26. FASTENERS / METAL CONNECTORS.

- A. ALL FASTENERS AND METAL CONNECTORS SHALL BE MANUFACTURED BY SIMPSON STRONG TIE AND INSTALLED PER THE MANUFACTURES SPECIFICATIONS AND INSTRUCTIONS.

- B. THESE FASTENERS DO NOT INCLUDE TYPICAL NAILS AND SCREWS WHICH MAY BE MANUFACTURED BY OTHERS.

- C. FOLLOW ALL MANUFACTURES SPECIFICATIONS AND INSTRUCTIONS FOR ALL FASTENERS, METAL CONNECTIONS, SCREWS, NAILS, ETC. THAT ARE IN CONTACT WITH PRESSURE TREATED LUMBER.

27. DIMENSIONAL LUMBER :

- A. ALL LOAD BEARING WALLS SHALL BE SOUTHERN YELLOW PINE #2 OR BETTER GRADED AND STAMPED BY THE CERTIFYING AGENCY . IN ADDITION, ALL WOOD SHALL BE PRESSURE TREATED FOR EXTERIOR USE WHERE EXPOSED TO MOISTURE, PLACED WITHIN 12 INCHES OF SOIL OR IN CONTACT WITH CONCRETE OR MASONRY.

28. STRUCTURAL SHEATHING:

- A. ALL SHEATHING USED FOR EXTERIOR APPLICATIONS SHALL BE EXTERIOR GRADE AND ADA STAMPED AND VERIFYING ITS RATING.

29. MASONRY:

- A. CONCRETE MASONRY UNITS SHALL CONFORM WITH AMERICAN MASONRY INSTITUTE STANDARD 530
- B. CONCRETE MASONRY UNITS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI
- C. MORTAR SHALL BE OF TYPE M OR S GRAY MORTAR.

30. GROUT:

- A. ALL GROUT SHALL BE A FINE TYPE HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI UNLESS SPECIFICALLY SHOWN OTHERWISE BY A MANUFACTURER PURSUANT TO GROUT USE WITH ITS PRODUCTS.

31. REINFORCING STEEL :

- A. ALL REINFORCING STEEL SHALL BE ASTM GRADE 40 EXCEPT GRADE 60 SHALL BE USED FOR GRADE BEAMS, ALL LINTEL TYPES (I.E. PRECAST AND FIELD PREFORMED) COLUMNS UNLESS OTHERWISE SHOWN IN THE STRUCTURAL PLANS.

Pasco County Florida

REVIEWED FOR CODE COMPLIANCE

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STRUCTURAL ENGINEER NOTES



DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-576-6831

PLAN DATE	04-28-20
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BUILDING 10 RESERVE AT HUNTERS RIDGE PHASE TWO

HEREBY CERTIFY THAT I HAVE PERFORMED THE ATTACHED DESIGN TO COMPLY WITH 145 MPH ULTIMATE WIND LOADS AND IT IS IN COMPLIANCE WITH SECT. 301 OF THE 2017 FLORIDA RESIDENTIAL BUILDING CODE SEALED FOR STRUCTURE ONLY SIGNED: RICHARD E. ALLEN P.E. #568920

ALLEN ENGINEERING & CONSTRUCTION SERVICES RICH ALLEN PROFESSIONAL ENGINEER P.E. # 56920 C.A. # 9542 P.O. BOX 351 NEW PORT RICHEY, FL. 34656 727-542-6100 rcaltenpe@gmail.com

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32. STRUCTURAL STEEL AND CONNECTION ACCESSORY MATERIAL:
- A. I-BEAMS, FORMED STRUCTURAL STEEL, FLAT BAR OR PLATE SHALL BE ASTM GRADE A36 UNLESS STATED OTHERWISE.
 - B. ALL STRUCTURAL STEEL SHALL HAVE A MINIMUM OF TWO COATS OF PRIMER AND TWO COATS OF EPOXY AS A CORROSION PREVENTIVE. THE BUILDING CONTRACTOR MAY VARY FROM THIS SPECIFICATION WITH THE APPROVAL OF THE STRUCTURAL ENGINEER IF IT CAN BE DEMONSTRATED ANOTHER MEANS OF CORROSION CONTROL IS EQUALLY EFFECTIVE.
 - C. ALL WELDING OF STRUCTURAL STEEL SHALL BE MADE WITH E6070 TYPE ELECTRODES. THE DEPTH AND LENGTH FOR THE WELD SHALL BE SPECIFIED IN THE STRUCTURAL DESIGN FOR THE SPECIFIC CONNECTION.
33. VENTILATION:
- A. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR DETERMINING VENTILATION REQUIREMENTS OF CRAWL SPACES, FLOORS AND ATTICS NOR THE MEANS AND METHODS FOR IMPLEMENTING THESE REQUIREMENTS.
34. WATERPROOFING:
- A. ANY RENDERING OF NOTES OF WATERPROOFING MEASURES FOR BASEMENTS OR HALF BASEMENTS SHOWN IN THESE PLANS WHERE A SPECIFIC CONSTRUCTION DETAIL IS NOT SHOWN IN THE STRUCTURAL DESIGN IS AN ARCHITECTURAL ILLUSTRATION ONLY AND IS NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
 - B. CRICKETS ARE ASSOCIATED WITH THE ARCHITECTURAL FINISHES AND ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
35. FIRE RESISTANT DESIGN:
- A. FIRE RESISTANT DESIGN OF STRUCTURAL ELEMENTS SHALL BE INCIDENTAL TO THEIR STRUCTURAL DESIGN AND SHALL BE BASED ON UNDERWRITERS LABORATORY OR GYPSUM ASSOCIATION DESIGN FOR FIRE RATED FLOOR, WALL AND ROOF ASSEMBLIES.
36. FLOOD RESISTANT DESIGN:
- A. FLOOD RESISTANT DESIGN OF FLOOD RESISTANT DESIGN OF STRUCTURAL ELEMENTS SHALL BE INCIDENTAL TO THEIR STRUCTURAL DESIGN AND SHALL BE BASED ON THE REQUIREMENTS STATED IN TITLE 44 CFR SECTIONS 59 AND 60, AND ON THOSE OF THE INDIVIDUAL COMMUNITY RATING AGENCIES FOR THE GOVERNMENTAL JURISDICTION WHERE THE CONSTRUCTION IS TO BE DONE.
 - B. HOWEVER, THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR IDENTIFYING AND SHOWING ON THE PLANS THE FLOOD ZONE CATEGORY, BASE FLOOD ELEVATION, AND THE FLOOR AND STORY HEIGHTS OF THE BUILDING IN RELATION TO THE BASE FLOOD ELEVATION. THIS INFORMATION IS CONSIDERED ARCHITECTURAL AND SITE RELATED AND SHALL BE PROVIDED TO THE STRUCTURAL ENGINEER BY THE CONTRACTING CLIENT OR HIS AGENT.
37. SPECIAL CONSTRUCTION:
- I. ALUMINUM STRUCTURAL COLUMNS.
 - A. ANY ALUMINUM STRUCTURES SHOWN IN THESE PLANS SUCH AS PORCH AND POOL ENCLOSURES OR GUARDRAILS AND HANDRAILS ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
 - B. WHERE THE ALUMINUM STRUCTURE ATTACHES TO THE MAIN STRUCTURE OR IS INCORPORATED IN THE MAIN STRUCTURE, SHOP DRAWINGS FOR THESE STRUCTURES SHALL BE PROVIDED TO THE STRUCTURAL ENGINEER TO DETERMINE THEIR EFFECT ON THE MAIN STRUCTURE.
 - II. SWIMMING POOLS:
 - A. ANY SWIMMING POOL OR HOT TUBS SHOWN IN THESE PLANS ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL DESIGN.
 - III. FENCES AND RETAINING WALLS:
 - A. ANY RENDERING OF FENCES, RETAINING WALLS OR EXTERIOR PLANTERS WHERE A SPECIFIC STRUCTURAL DETAIL IS NOT SHOWN FOR THEIR CONSTRUCTION ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
 - IV. DRIVEWAYS AND WALKWAYS:
 - A. ANY DRIVEWAYS OR WALKWAYS SHOWN IN THESE PLANS ARE FOR ARCHITECTURAL ILLUSTRATION PURPOSES ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

The info was calculated using the provisions of the 2017 Florida Building Code.

Floor and Roof Live Loads

Attics:	20 psf w/ storage, 10 psf w/o storage
Habitable Attics, Bedroom:	30 psf
All Other Rooms:	40 psf
Garage:	40 psf
Roofs:	20 psf

Wind Design Data

Ultimate Wind Speed:	145 mph	Nominal Wind Speed:	112 mph
Risk Category:	II	Wind Exposure:	B
Enclosure Classification:	Enclosed	End Zone Width:	4.00 ft.
Internal Pressure Coefficient:	0.18 +/-		

Components and Cladding Design Pressures	Roof Zone 1:	+21.8 psf max., -34.7 psf min.
	Roof Zone 2:	+21.8 psf max., -60.5 psf min.
	Roof Zone 3:	+21.8 psf max., -89.5 psf min.
	Roof at Zone 2 Overhangs:	-70.6 psf min.
	Roof at Zone 3 Overhangs:	-118.8 psf min.
	Wall Zone 4:	+37.9 psf max., -41.1 psf min.
	Wall Zone 5:	+37.9 psf max., -50.7 psf min.
	16' X 7' OHGD:	+32.1 psf max., -35.9 psf min.

The Ultimate Wind Speed was used to determine the above Component and Cladding Design Pressures.

All exterior glazed openings shall be protected from wind-borne debris as per Section 1609.1.2 of the code.

The site of this building is not subject to special topographic wind effects as per Section 1609.1.1.1 of the code.

Geotechnical Information

Design Soil Load-Bearing Capacity:	2,000 psf
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Flood Design Data

Flood Zone:	X
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This table was created using Windload Calculator Plus (Software available at www.windecals.com)

WIND LOAD DESIGN DATA



DEEB FAMILY HOMES, LTD.
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NEW PORT RICHEY, FL. 34655
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BUILDING 10

RESERVE AT HUNTERS RIDGE PHASE TWO

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SIGNED: RICHARD E. ALLEN P.E. #66920

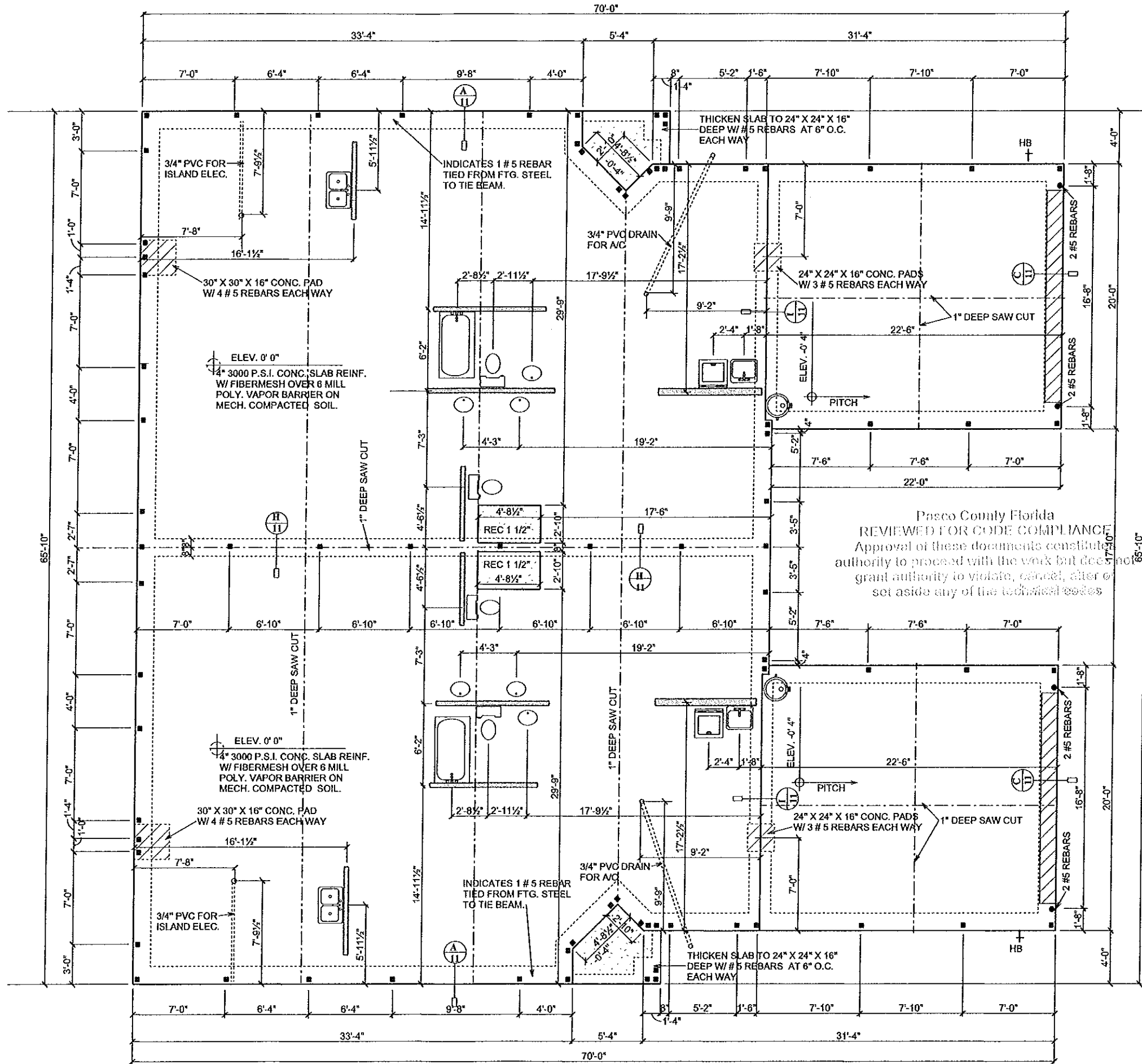
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QUAIL MODEL

ALLEN ENGINEERING & CONSTRUCTION SERVICES
RICH ALLEN PROFESSIONAL ENGINEER
P.E. # 56920 C.A. # 9542

P.O. BOX 351
NEW PORT RICHEY, FL. 34656
727-842-6100
richallenpe@gmail.com

- NOTES**
- 1) THE FOUNDATION SYSTEM FOR THIS PLAN IS DESIGNED FOR A MINIMUM ALLOWABLE SOIL BEARING PRESSURE OF 2,000 P.S.F. WITH NO SOILS REPORT OR INFORMATION PROVIDED.
 - 2) FOOTINGS TO BEAR MIN. 12" BELOW GRADE.
 - 3) FOOTINGS TO BEAR ON UNDISTURBED SOIL OR FILL COMPACTED TO 95% MOD. PROCTOR BETWEEN LESS THAN 12" LIFTS.
 - 4) ALL BEARING SOILS TO BE FREE OF DEBRIS AND ORGANIC MATERIAL.
 - 5) REFER TO STRUCTURAL ENGINEER NOTES.



SLAB PLAN

SCALE 1/8" = 1'-0"

1

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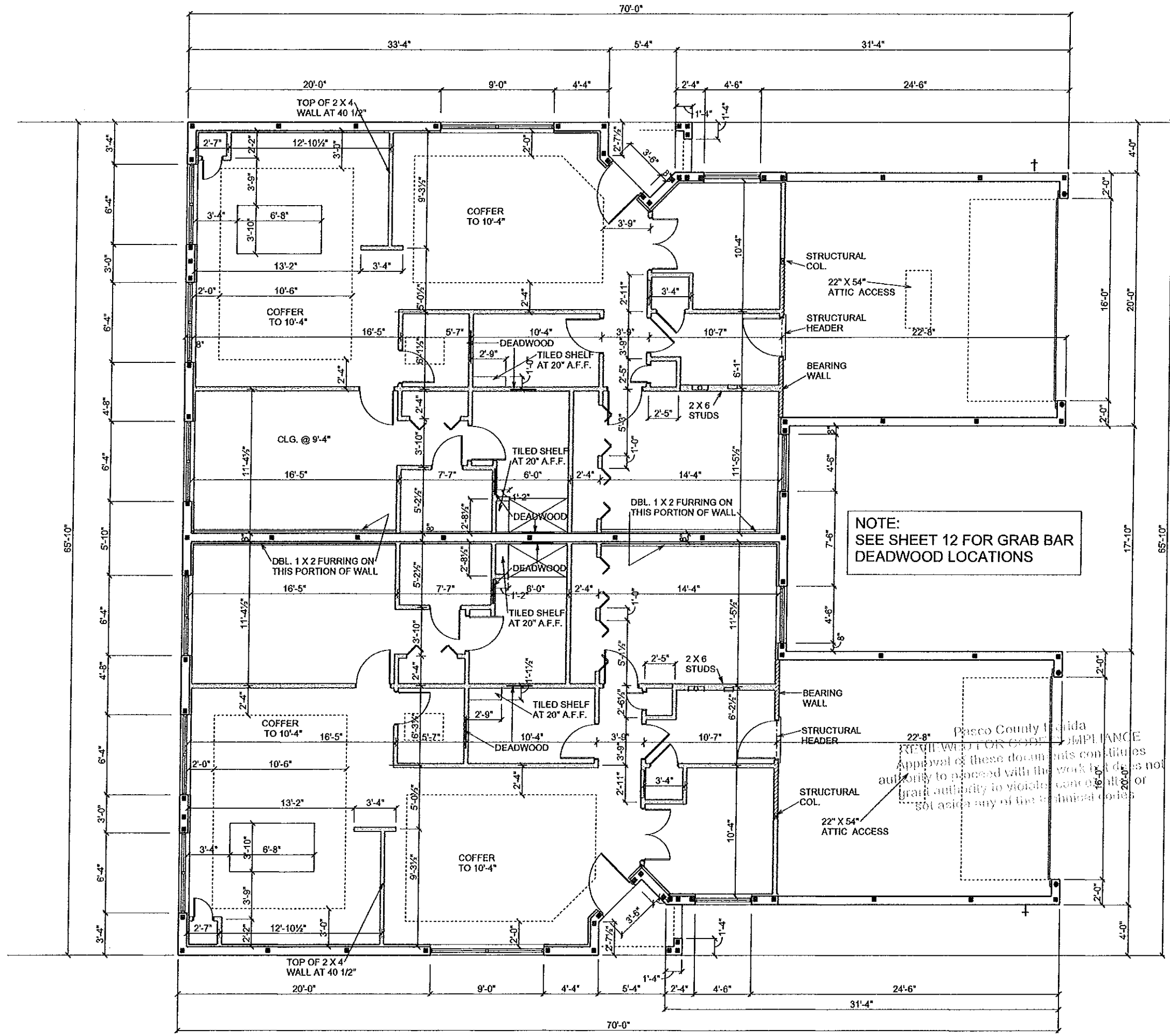
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DIMENSION PLAN SCALE 1/8" = 1'-0" **A.E.C.S. 20046** **QUAIL MODEL**

3

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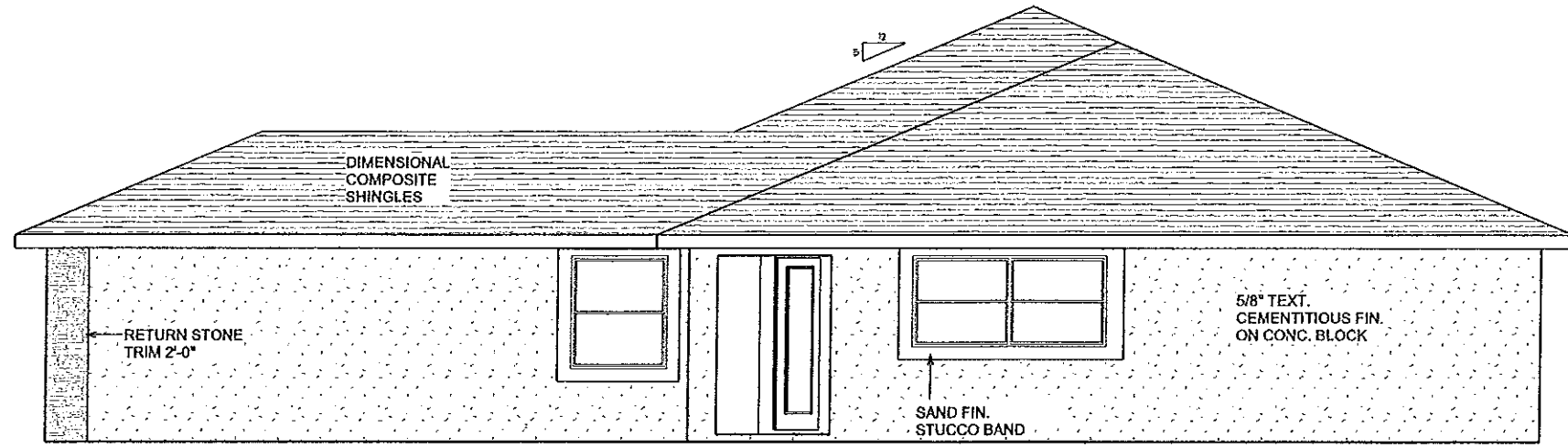
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 SIGNED: *[Signature]* 5/13/2020
 RICHARD E. ALLEN P.E. #56920

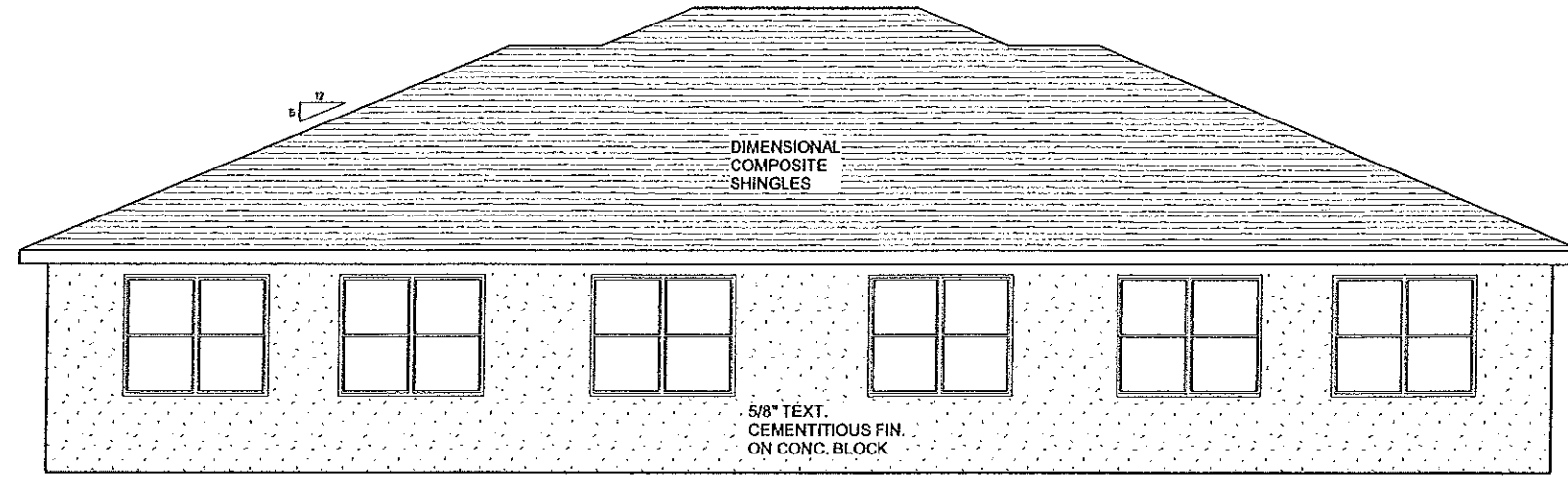
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NOTE:
 SEE SHEET 12 FOR GRAB BAR DEADWOOD LOCATIONS

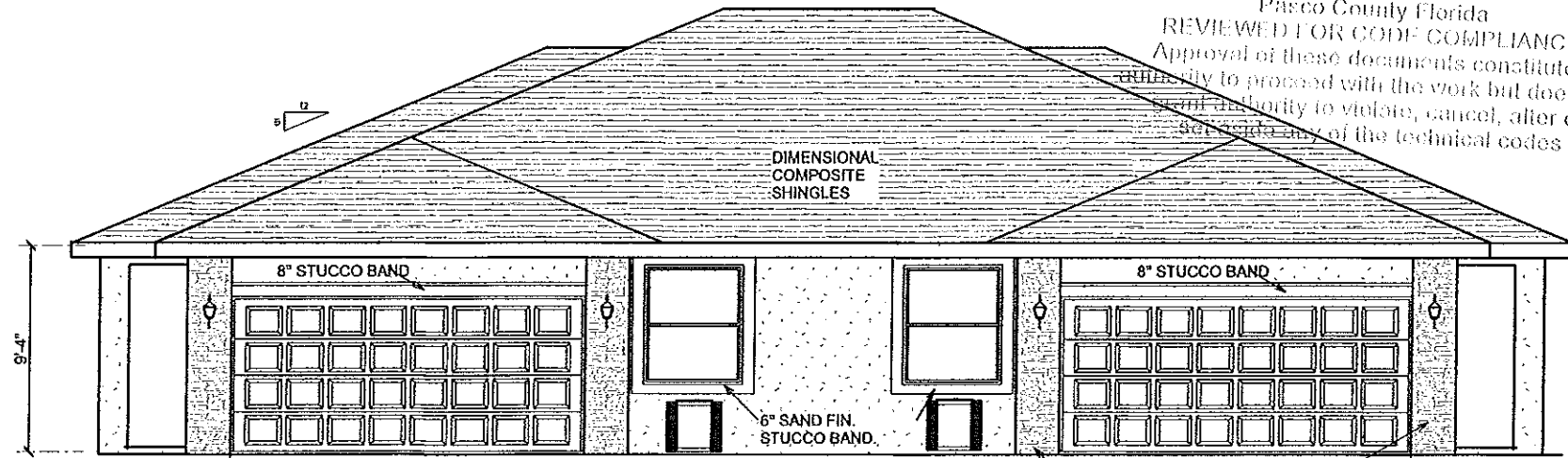
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SIDE ELEVATION



REAR ELEVATION



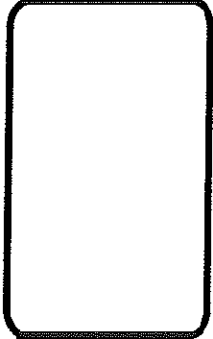
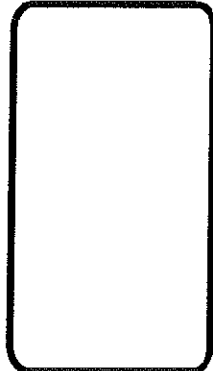
FRONT ELVATION

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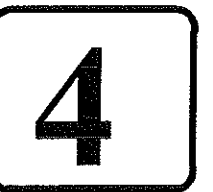
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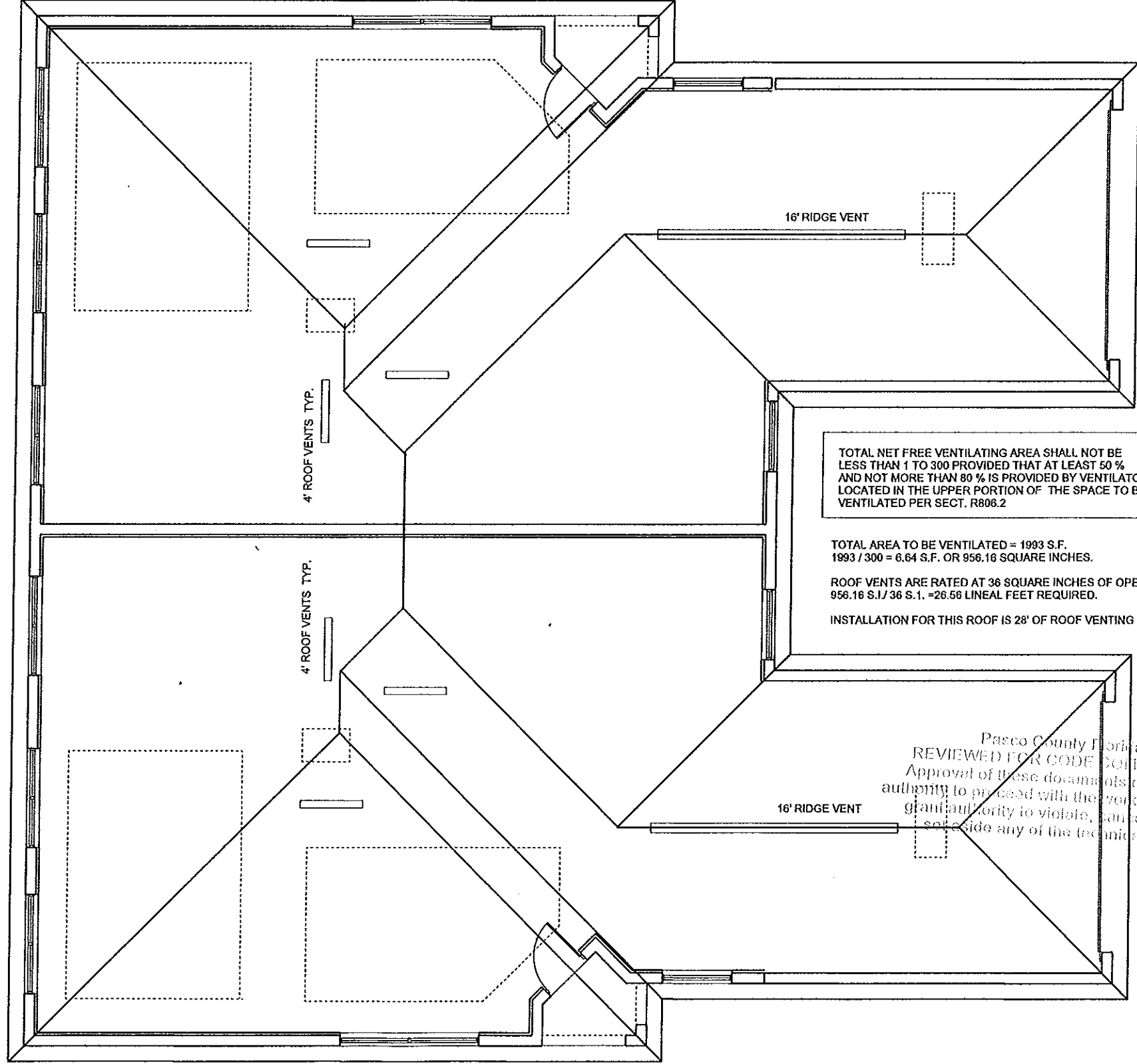


BUILDING 10
 RESERVE AT HUNTERS RIDGE
 PHASE TWO

PLAN DATE
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 HOMES, L.T.D.
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ROOF PLAN

SCALE 1/8" = 1'-0"

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UNLESS OTHERWISE NOTED:
 1. ELECTRICAL OUTLET HEIGHTS MEASURED FROM FINISHED FLOOR TO CENTERLINE OF THE BOX TO BE 18" A.F.F. (GENERAL)

KITCHEN 42"
 BATHROOM 42"
 LAUNDRY 36" WASHER/ 24" DRYER/ WALL OUTLETS 45"
 EXTERIOR WATERPROOF @ 12"
 GARAGE GFI @ 45"
 RANGE 220V @ 4"

2. ALL TRIM PLATES AND DEVICES TO GANGED WHERE POSSIBLE

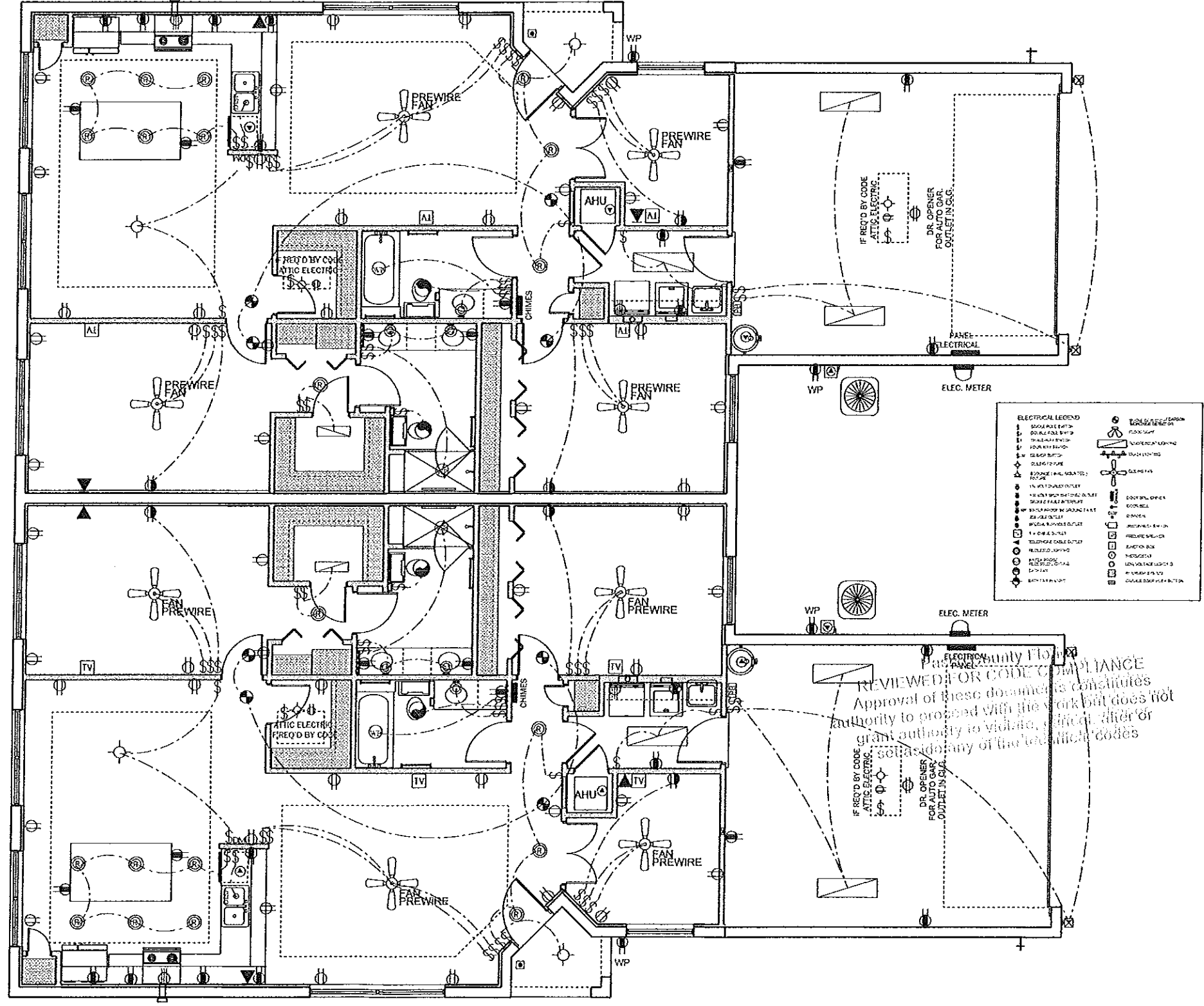
3. ELECTRICAL SWITCHES TO BE AT 42" CENTERLINE A.F.F.

4. ELECTRICAL PLAN IS INTENDED FOR BID PURPOSES ONLY. ALL WORK SHALL BE DONE IN STRICT ACCORDANCE WITH THE NATIONAL ELECTRICAL CODE, LATEST EDITION BY A LICENSED ELECTRICAL CONTRACTOR WHO SHALL BE RESPONSIBLE FOR THE INSTALLATION & SIZING OF ALL ELECTRICAL, WIRING & ACCESSORIES.

5. SMOKE DETECTORS SHALL BE IN ACCORDANCE WITH THE FLORIDA BUILDING CODE, R314 AND SHALL BE INTERCONNECTED

6. PROVIDE AFCI (ARC FAULT INTERRUPTERS) IN ALL AREAS PER NEC, SECTION 210-12

7. ALL RECEPTALS TO BE TAMPER PROOF PER SECT. 406.11



ELECTRICAL PLAN

SALE 1/8" = 1'-0"

A.E.C.S. 20046 QUAIL MODEL

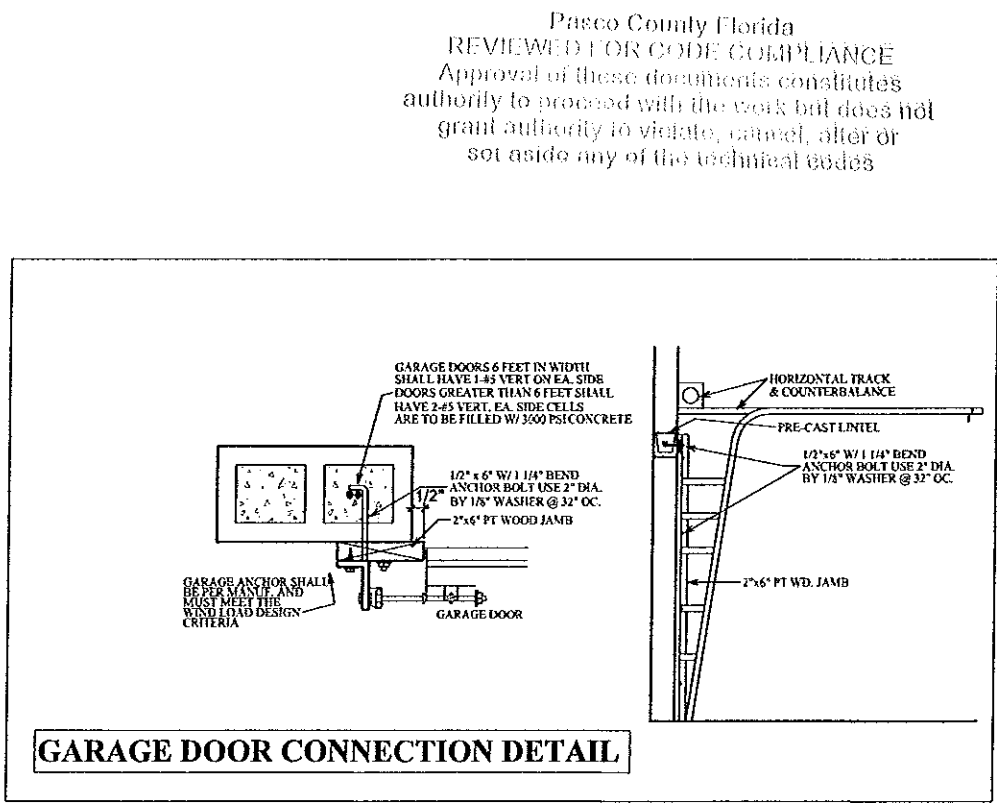
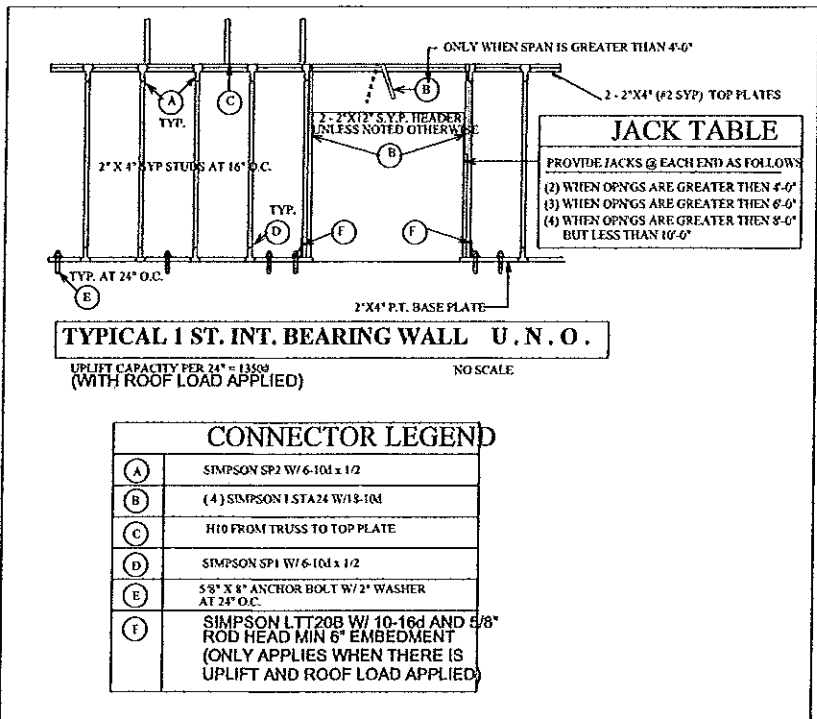
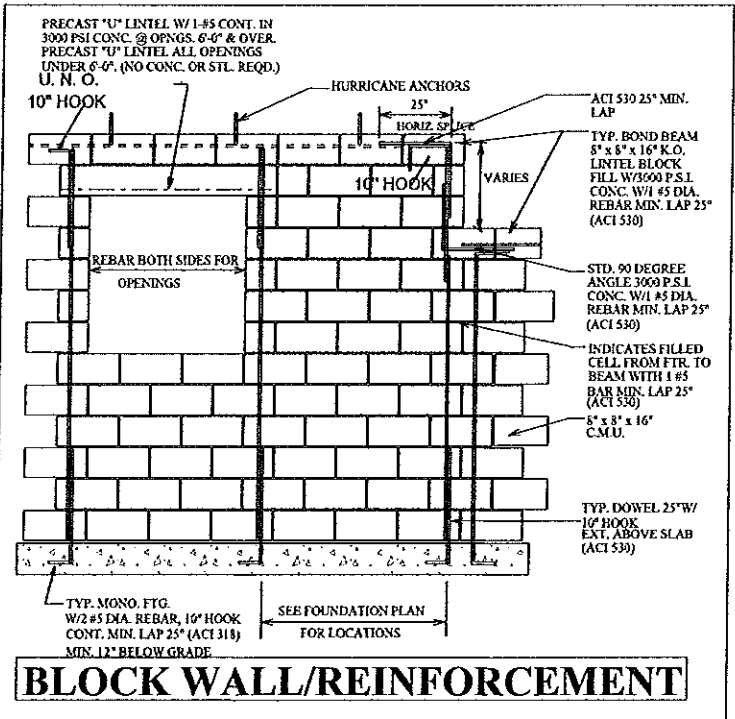
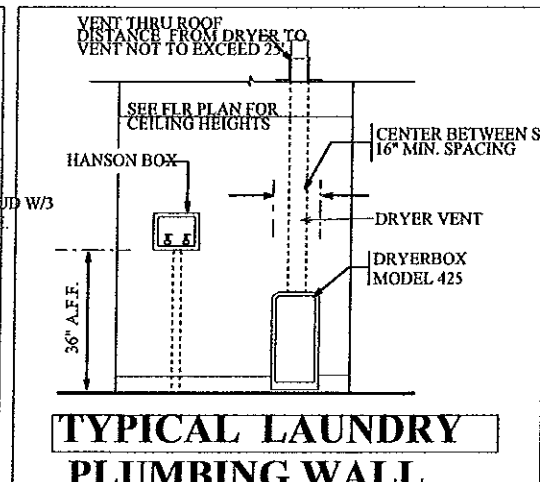
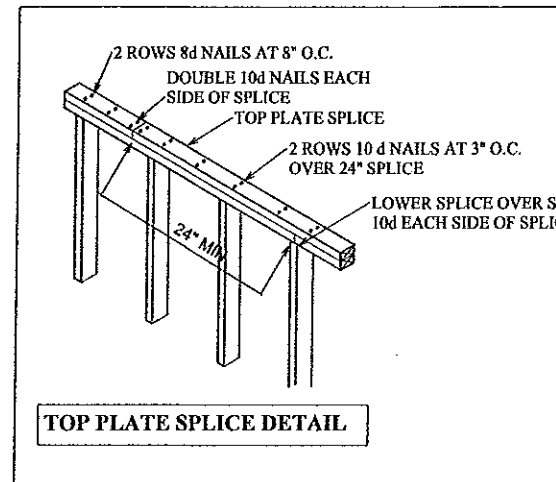
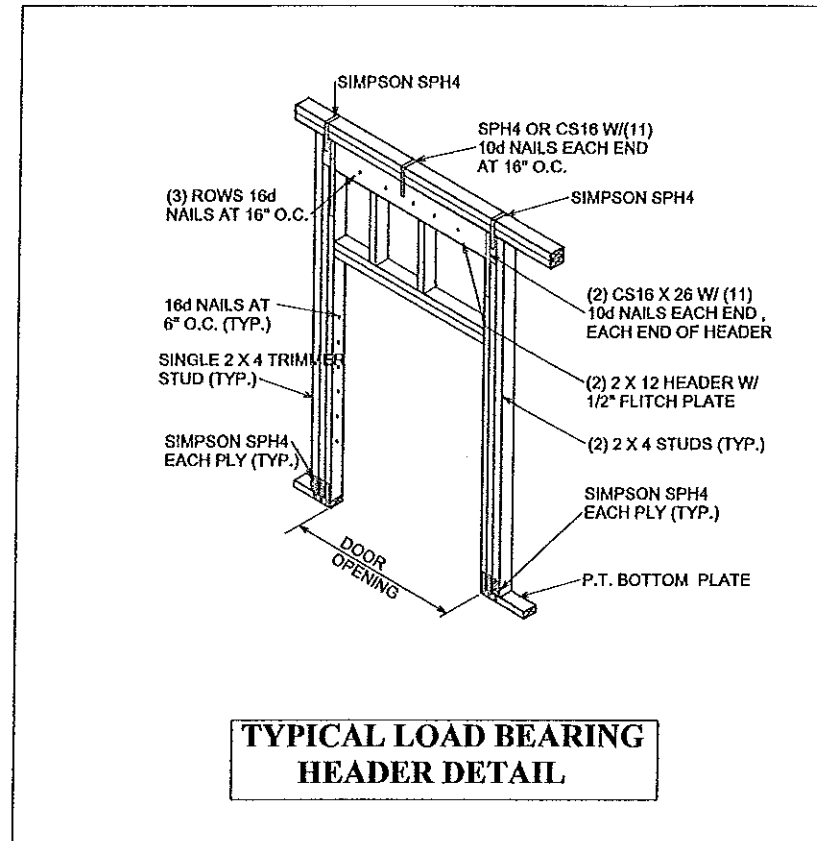
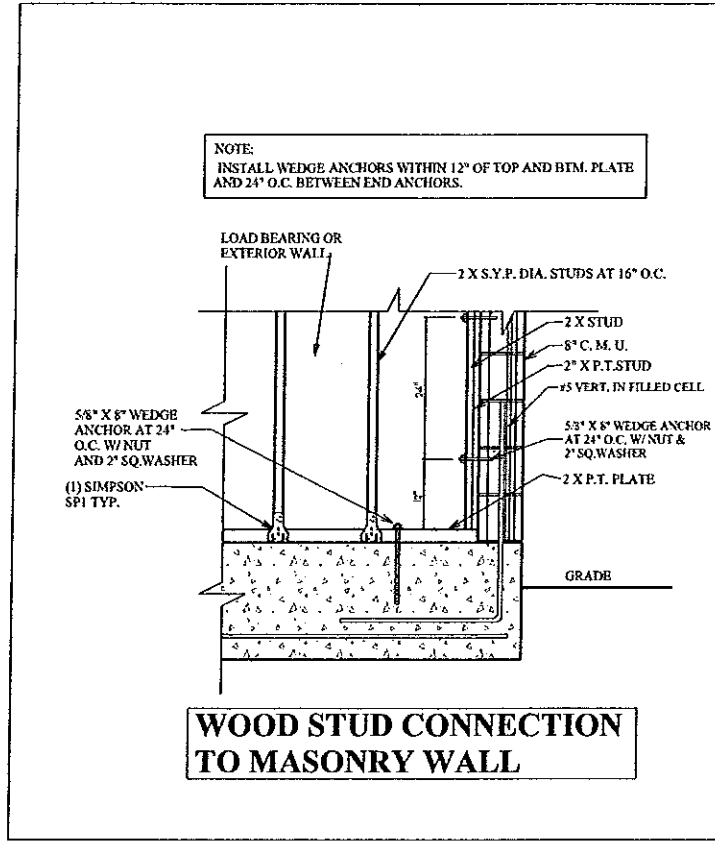
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BUILDING 10
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 PHASE TWO



CONST. DETAILS

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QUAIL MODEL

BUILDING 10

RESERVE AT HUNTERS RIDGE PHASE TWO

PLAN DATE

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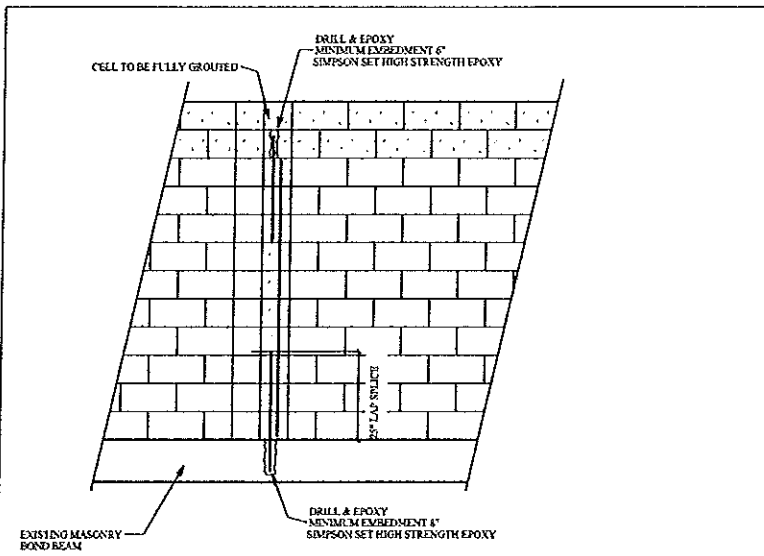
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8

ALLEN ENGINEERING & CONSTRUCTION SERVICES
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5/13/20

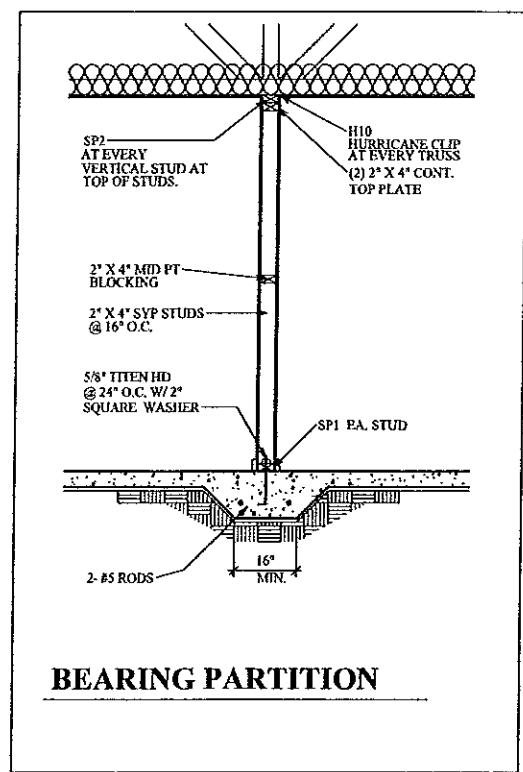


TYP. RETROFIT VERT. DOWEL CONDITION

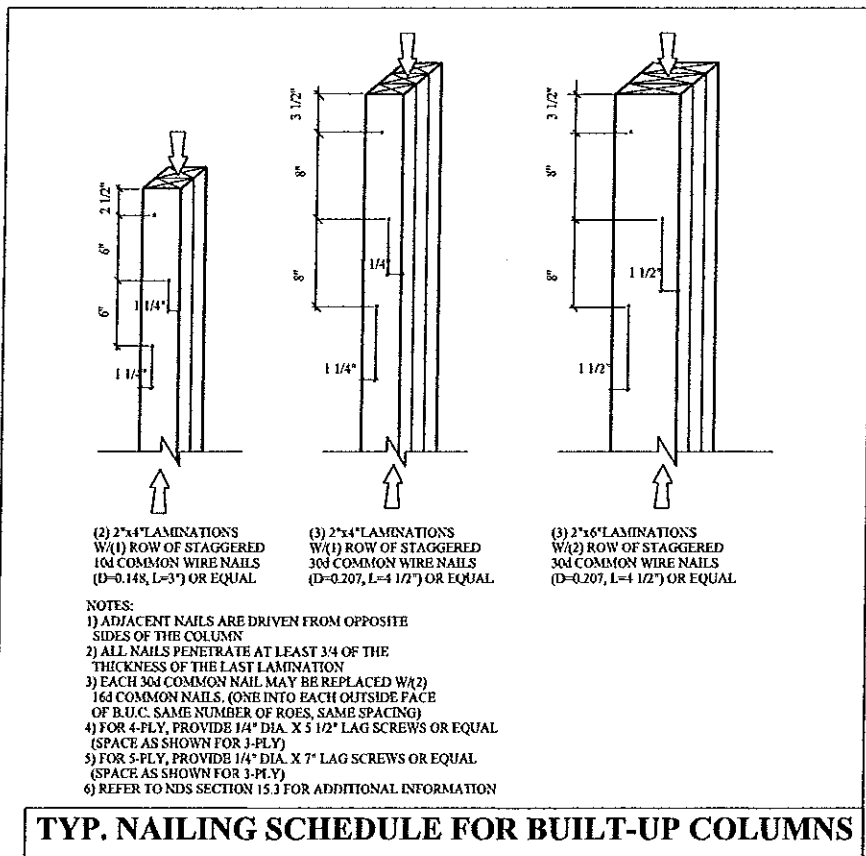
NOTE:
MISSING DOWELS: WHERE FOOTING DOWELS ARE PLACED INCORRECTLY OR MISTAKENLY ELIMINATED, REPLACE DOWEL AT PROPER LOCATION W/ GRADE 40 #5 BAR. INSTALL IN SLAB W/ 8" MINIMUM EMBEDMENT, USE EPOXY GROUT.

MISSING ANCHOR BOLTS AT BEARING WALL:

- EXTERIOR BEARING WALL:
IN ADDITION TO THE GENERAL PLACEMENT REQUIREMENTS:
- 1) 5/8" DIAMETER x 6" EMBEDMENT SIMPSON TITEN HD ANCHORS SPACED A MAXIMUM OF 24" O.C.
- INTERIOR BEARING WALL:
IN ADDITION TO THE GENERAL PLACEMENT REQUIREMENTS:
- 1) 5/8" DIAMETER x 6" EMBEDMENT SIMPSON TITEN HD ANCHORS SPACED A MAXIMUM OF 24" O.C. IF RESISTING UPLIFT LOADS OR 3 1/2" EMBEDMENT AT 48" O.C. IF RESISTING GRAVITY LOADS

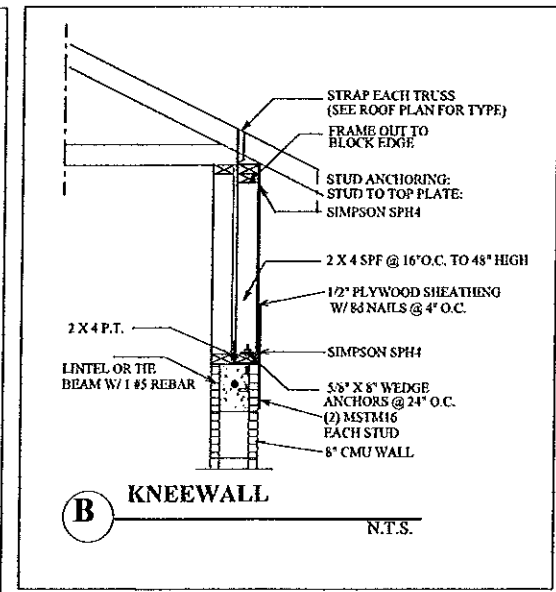


BEARING PARTITION



- NOTES:
- 1) ADJACENT NAILS ARE DRIVEN FROM OPPOSITE SIDES OF THE COLUMN
 - 2) ALL NAILS PENETRATE AT LEAST 3/4 OF THE THICKNESS OF THE LAST LAMINATION
 - 3) EACH 30d COMMON NAIL MAY BE REPLACED W/ 2) 16d COMMON NAILS. (ONE INTO EACH OUTSIDE FACE OF B.U.C. SAME NUMBER OF ROWS, SAME SPACING)
 - 4) FOR 4-PLY, PROVIDE 1/4" DIA. X 5 1/2" LAG SCREWS OR EQUAL (SPACE AS SHOWN FOR 3-PLY)
 - 5) FOR 5-PLY, PROVIDE 1/4" DIA. X 7" LAG SCREWS OR EQUAL (SPACE AS SHOWN FOR 3-PLY)
 - 6) REFER TO NDS SECTION 15.3 FOR ADDITIONAL INFORMATION

TYP. NAILING SCHEDULE FOR BUILT-UP COLUMNS



(B) KNEEWALL N.T.S.

Pasco County Florida
REVIEWED FOR CODE COMPLIANCE
Approval of these documents constitutes
authority to proceed with the work but does not
authority to violate, cancel, alter or
set aside any of the technical codes

Design No. U905
March 11, 2016

Bearing Wall Rating — 2 HR.
Nonbearing Wall Rating — 2 HR.

FIRE RESISTANCE RATING - 2 HOURS

This design was evaluated using a load design method other than the Limit States Design Method (e.g., Working Stress Design Method). For jurisdictions employing the Limit States Design Method, such as Canada, a load restriction factor shall be used — See Guide **BXUV** or **BXUVZ**

* Indicates such products shall bear the UL or cUL Certification Mark for jurisdictions employing the UL or cUL Certification (such as Canada), respectively.

1. Concrete Blocks* — Various designs. Classification D-2 (2 hr).
See Concrete Blocks category for list of eligible manufacturers.
2. Mortar — Blocks laid in full bed of mortar, nom. 3/8 in. thick, of not less than 2-1/4 and not more than 3-1/2 parts of clean sharp sand to 1 part Portland cement (proportioned by volume) and not more than 50 percent hydrated lime (by cement volume). Vertical joints staggered.
3. Portland Cement Stucco or Gypsum Plaster — Add 1/2 hr to classification if used. Where combustible members are framed in wall, plaster or stucco must be applied on the face opposite framing to achieve a max. Classification of 1-1/2 hr. Attached to concrete blocks (Rem 1).
4. Loose Masonry Fill — If all core spaces are filled with loose dry expanded slag, expanded clay or shale (Rotary Kiln Process), water repellent vermiculite masonry fill insulation, or silicone treated perlite loose fill insulation add 2 hr to

CONSTRUCTION DETAILS

A.E.C.S. 20046 QUAIL MODEL

ALLEN ENGINEERING & CONSTRUCTION SERVICES
RICH ALLEN PROFESSIONAL ENGINEER
P.E. # 56920 C.A. # 9542
P.O. BOX 351
NEW PORT RICHEY, FL. 34656
727-842-6100
richallenpe@gmail.com

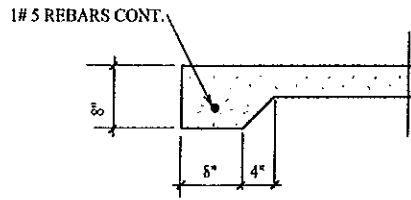
I HEREBY CERTIFY THAT I HAVE PERFORMED THE ATTACHED DESIGN TO COMPLY WITH 145 MPH ULTIMATE WIND LOADS AND IT IS IN COMPLIANCE WITH SECT. 301 OF THE 2017 FLORIDA RESIDENTIAL BUILDING CODE
SEALED FOR STRUCTURE ONLY
SIGNED: *[Signature]*
RICHARD E. ALLEN P.E. #56920

BUILDING 10
RESERVE AT HUNTERS RIDGE
PHASE TWO

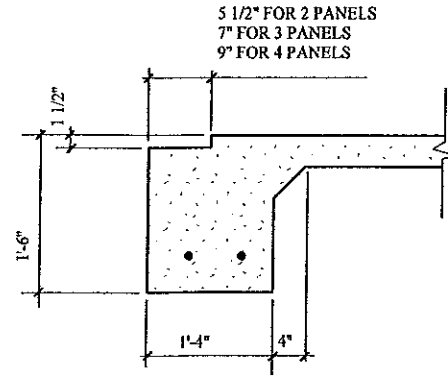
PLAN DATE
04-28-20

DEEB FAMILY HOMES, L.T.D.
9400 RIVER CROSSING BLD.
NEW PORT RICHEY, FL. 34655
727-376-6831

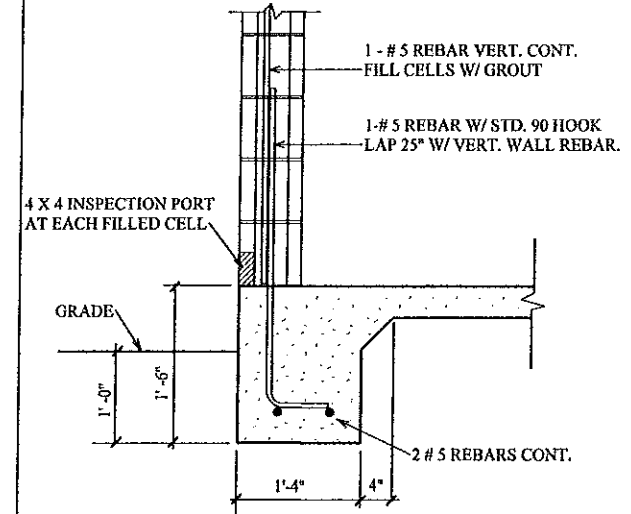
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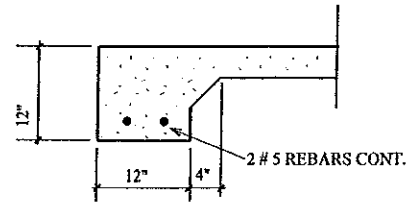
8" THICKENED SLAB (J)



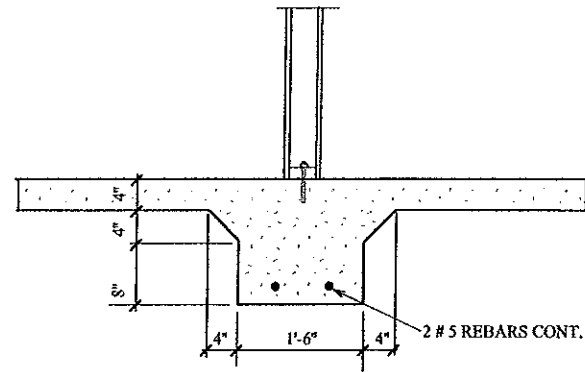
SLIDING GLASS DR. RECESS (D)



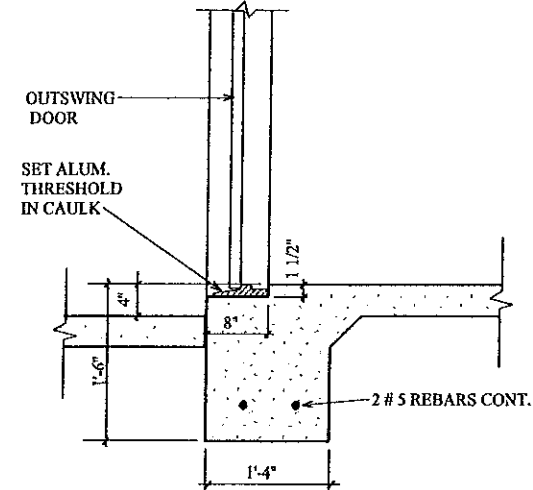
TYPICAL ONE STORY (A)



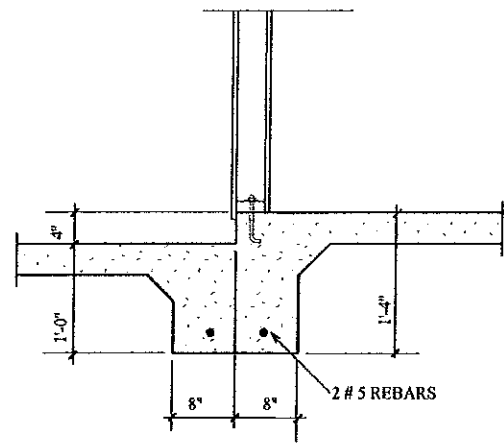
12" THICKENED SLAB (K)



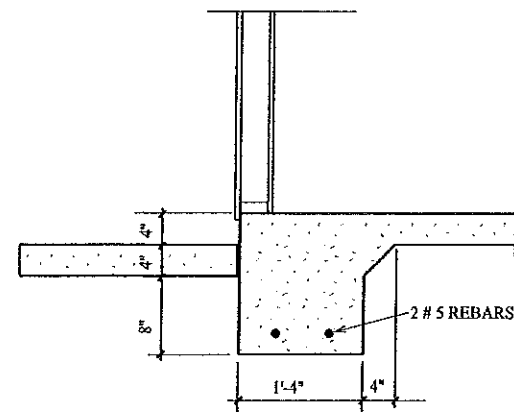
INTERIOR BEARING FTG. (H)



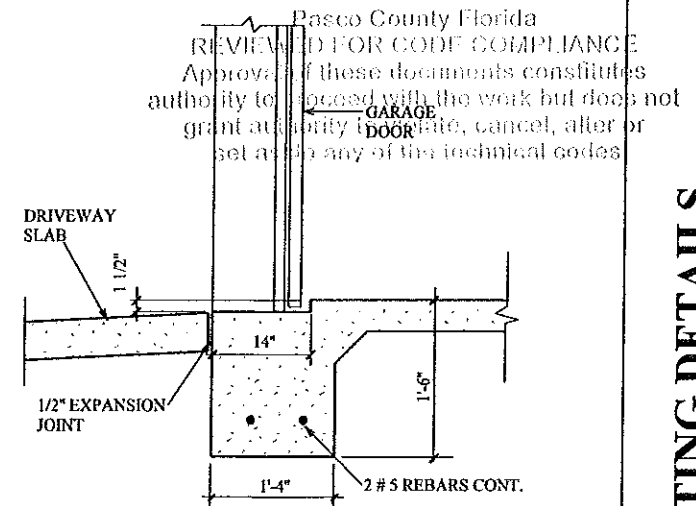
EXTERIOR DOOR RECESS (B)



BEARING GARAGE STEP (I)



NON-BRG. GARAGE STEP (F)



GARAGE DOOR RECESS (C)

FOOTING DETAILS

QUAIL MODEL
ALLEN ENGINEERING & CONSTRUCTION SERVICES
 RICH ALLEN PROFESSIONAL ENGINEER
 P.E. # 56920 C.A. # 9542
 P.O. BOX 351
 NEW PORT RICHEY, FL. 34656
 727-842-6100
 richallenpe@gmail.com

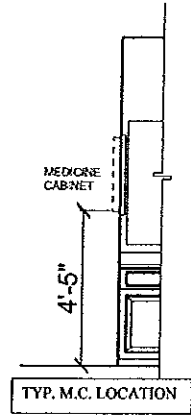
A.E.C.S. 20046
 I HEREBY CERTIFY THAT I HAVE PERFORMED THE ATTACHED DESIGN TO COMPLY WITH 145 MPH ULTIMATE WIND LOADS AND IT IS IN COMPLIANCE WITH SECT. 307 OF THE 2017 FLORIDA RESIDENTIAL BUILDING CODE SEALED FOR STRUCTURE ONLY
 SIGNED: *[Signature]*
 RICHARDO ALLEN, P.E. #56920

BUILDING 10
 RESERVE AT HUNTERS RIDGE
 PHASE TWO

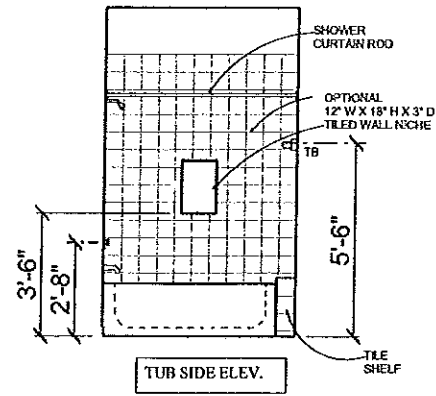
PLAN DATE
 04-28-20

DEEB FAMILY HOMES, LTD.
 9400 RIVER CROSSING BLD.
 NEW PORT RICHEY, FL. 34655
 727-376-6831

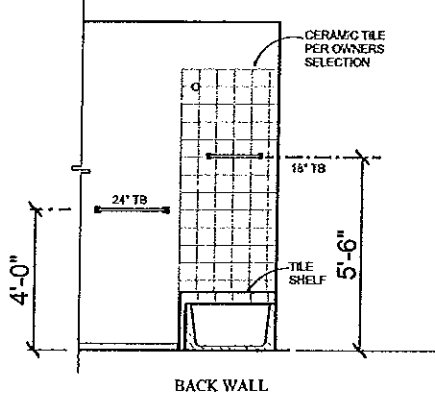
11



TYP. M.C. LOCATION

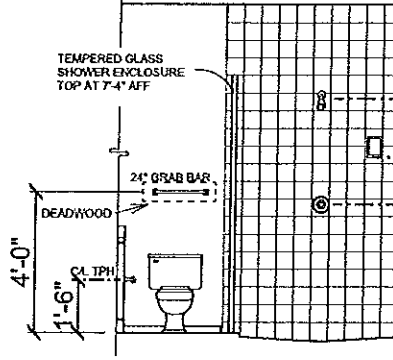


TUB SIDE ELEV.

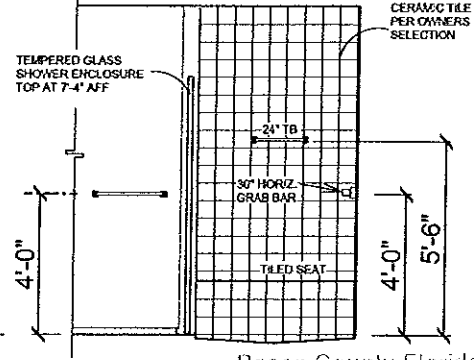


BACK WALL

BATHROOM TUB ELEVATIONS 'C-C' (WITH TUB SHELF)

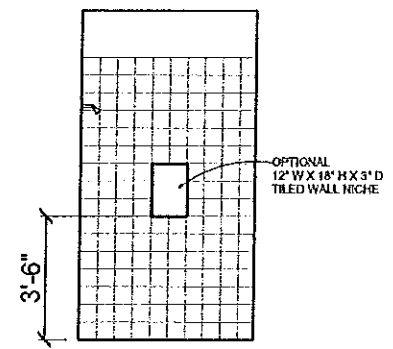


SHOWER HEAD WALL

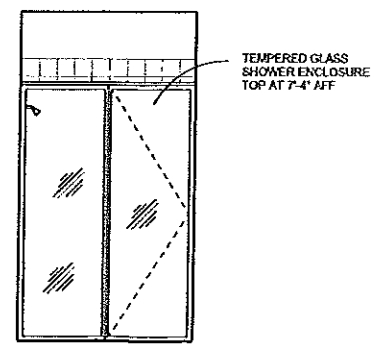


BACK WALL

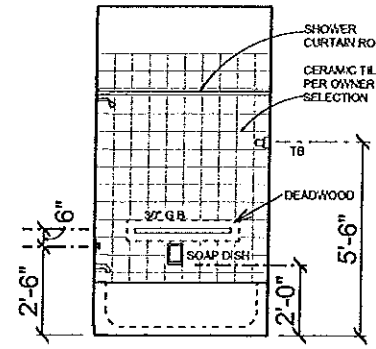
BATHROOM SHOWER ELEVATIONS (A-A)



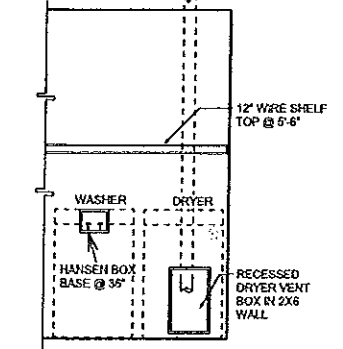
SHOWER SIDE ELEV.



SHOWER SIDE ENCL. ELEV.



TUB SIDE ELEV.



LAUNDRY ELEVATION

Pasco County Florida
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 no warranty or guarantee of accuracy and
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TYPICAL INTERIOR BATHROOM DETAILS

A.E.C.S. 20046 QUAIL MODEL

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PLAN DATE
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