13/5/19 (Shoner char quot Suite Both) need sents 13/18/19 (septed sheets, shonerchar) 3/8/19 D 1 4/25/19 (Bearing woll)

GENERAL NOTES:

THE FOLLOWING TECHNICAL CODES SHALL APPLY: 2017 FLORIDA BUILDING CODE, PLUMBING, MECHANICAL, FUEL GAS, ENERGY EFFICIENCY, ACCESSIBILITY, AND NATIONAL FLECTRICAL CODES NEC 2014

- 3.5 GALLONS
- PUBLIC FACILITIES 0.5 G.P.M. PRIVATE FACILITIES 2.2 G.P.M. SHOWER HEADS 2.5 G.P.M.

VTR LOCATIONS ARE APPROXIMATE AND MAY CHANGE DUE TO JOBSITE CONDITIONS

- WITH THE 2017 FBC.
- CHIMNEY & FIREPLACE
- ☐ EGRESS WINDOWS
- 4. ALL OPENINGS SHALL COMPLY WITH 2017 FBC WIND LOADS AS STATED BELOW, ATTACHMENTS OF WINDOWS. DOORS, SLIDING GLASS DOORS AND O.H. GARAGE DOORS ARE DELEGATED THE MANUFACTURER OF THESE ITEMS. THE MANUFACTURER OF THESE ITEMS SHALL SUBMIT ATTACHMENTS TO ENGINEER OF RECORD FOR REVIEW PRIOR TO INSTALLATION. SEE ATTACHED SPECIFICATION SHEETS FOR MANUFACTURERS DESIGN CRITERIA AND INSTALLATION METHODS FOR WINDOWS, DOORS, SLIDING GLASS DOORS, OVERHEAD GARAGE DOORS, AND ROOFING.
- 5. ALL DOORS INTERIOR & EXTERIOR ARE 8'0" UNLESS OTHERWISE NOTED ALL SHOWER ENCLOSURES TO BE

- 1. TANK TYPE WATER CLOSET VOLUME
- 2. WALL MOUNT WATER CLOSET VOLUME

3. WATER - FLOW RATE.

THE FOLLOWING SHALL COMPLY

- O PORCHES AND BALCONIES
- A HANDRAILS
- C GUARDRAILS
- C STAIRS

- TEMPERED GLASS
- ALL WINDOWS WITHIN 24" OF DOORS (INTERIOR & EXTERIOR) AND WITHIN 18" OFF FLR TO BE TEMPERED GLASS



IN DESIGN IAN A.I.B.D. COD SO. HEY, FL MASS

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AL ROBBI.
SENT CONNEWO
NEW POOF NICH.
(TZ) SIN CATAL
MALICALISTOPOPIUM



INDEX OF DRAWINGS

SHEET

31 \$3 **S4** 4A 10 11 12 **ELECTRICAL RISERS**

TITLE **COVER SHEET** STRUCTURAL ENGINEER NOTES STRUCTURAL ENGINEER NOTES STRUCTURAL ENGINEER NOTES WIND LOAD DESIGN DATA FOUNDATION PLAN FLOOR PLAN NOTES DIMENSION PLAN **EXTERIOR ELEVATIONS ENTRY TOWER DETAILS** EXTERIOR ELEVATIONS **ROOF PLAN** TRUSS PLAN **ELECTRICAL PLAN** CONSTRUCTION DETAILS CONSTRUCTION DETAILS TYPICAL WALL SECTIONS TYPICAL FOOTING DETAILS TYPICAL BATH DETAILS

SHEET



DEEB FAMILY
HOMES, LTD.
9400 RIVER CROSSING BLD.
NEW PORT HICHEY, FL. 34655
777-775-6831

MODEL

H

COVE

DUNEDIN COVI

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PLAN DATE

DUE TO SPACE LIMITATIONS IN THIS 11"X 17" PLAN FORMAT, AND TO ELIMINATE CLUTTER AND TEXT

NOTICE TO SUBCONTRACTORS:

READABILITY ISSUES, SOME DETAILS AND NOTATIONS MAY OR MAY NOT BE LOCATED ON THE SAME SHEETS OR IN THE SAME LOCATIONS AS PROVIDED FOR BY OTHER CONTRACTORS OR ARCHITECTS. IT WOULD BE IN YOUR BEST INTREST TO REVIEW THESE PLANS AND LOCATE THE APPROPORIATE INFORMATION REQUIRED TO COMPLETE YOUR SPECIFIC PORTION OF THE JOB BEFORE BEGINNING CONSTRUCTION.

IT IS THE INTENT OF THIS DESIGNER THAT THESE PLANS ARE ACCURATE AND ARE CLEAR ENOUGH FOR THE LICENSED PROFESSIONAL TO CONSTRUCT THIS PROJECT. IN THE EVENT THAT SOMETHING IS UNCLEAR OR NEEDS CLARIFICATION STOP, AND CALL THE DESIGNER LISTED IN THIS TITLE PAGE. IT IS THE RESPONSIBILITY OF THE LICENSED PROFESSIONAL THAT IS CONSTRUCTING THIS PROJECT TO FULLY REVIEW THESE DOCUMENTS BEFORE CONSTRUCTION BEGINS AND ANY AND ALL CORRECTIONS, IF NEEDED, TO BE MADE BEFORE ANY WORK IS DONE.

NOTICE TO BUILDER

WINDOW INSTALLATION NOTES:

WINDOWS MUST BE FASTENED INTO STRUCTURAL MEMBERS PER MFG'S, DETAIL REQUIREMENTS PER DESIGN CRITERIA NOTED ON THESE DRAWINGS. 2. WINDOWS ARE NOT IMPACT RESISTANT TYPE. STORM SHUTTERS OR PANELS ARE REQUIRED. D. ROOF, WALLS AND WINDOW FASTENINGS MUST BE ENGINEERED AND SPECIFIED FOR CUMULATIVE INTERNAL PRESSURE AND EXTERNAL NEGATIVE (SUCTION) PRESSURES WHICH VARIES ACCORDING TO AREAS AS NOTED IN THE DESIGN CRITERIA AS NOTED ON PAGE \$4.

ADMINISTRATIVE

1. THE ENGINEERING FIRM FOR THIS STRUCTURAL DESIGN IS ALLEN ENGINEERING AND CONSTRUCTION SERVICES,INC. HEREIN REFERRED TO AS " AECS OR " A.E.C.S ". 2. THE ENGINEER FOR THIS STRUCTURAL DESIGN IS

RICHARD E. ALLEN, PE. HEREIN REFERRED TO AS "STRUCTURAL

3. THE STRUCTURAL ENGINEER DESIGN NOTES ARE PART OF THE STRUCTURAL DESIGN AND ARE TO BE TAKEN AS TYPICAL REQUIREMENTS UNLESS NOTED OTHERWISE, "UNO" IN THE STRUCTURAL PLANS AND STRUCTURAL DETAILS. 4. THE DESIGN SHOWN IN THESE PLANS CONFORM TO THE

4. THE DESIGN SHOWN IN THESE PLANS CONFORM TO THE STRUCTURAL PROVISIONS OF THE CHAPTER IS OF THE FLORIDA BUILDING CODE, SECTION R301 OF THE FLORIDA RESIDENTIAL BUILDING CODE 2017, THE SECTIONS TITLED "STRUCTURAL" OF THE FLORIDA EXISTING BUILDING CODE 2017.

5. THE PURPOSE OF THESE PLANS IS TO OBTAIN A BUILDING PERMIT AND FOR SUBSEQUENT CONSTRUCTION OF THE DESIGN AS SHOWN. THESE PLANS ARE TO BE CONSIDERED VOID IF WORK COMMENCES PRIOR TO A PERMIT BEING ISSUED, A CHANGE IN THE BUILDING CODE OCCURS PRIOR TO THE PLANS BEING SUBMITTED FOR PERMIT OR ATTER BY MADEIR OF THE ASS. THE BUILDING CODE OCCURES PRIOR TO THE PLANS BEING SUBMITTED FOR PERMIT OR AFTER SIX MONTHS OF THE DATE THAT THESE PLANS ARE SIGNED AND SEALED WITHOUT BEING SUBMITTED FOR PERMITTING, WHICHEVER OCCURES FIRST, ONCE A BUILDING PERMIT HAS BEEN ISSUED BASED ON THESE PLANS, THE BUILDING DEPARTMENT IS NOT AUTHORIZED TO REISSUE OR TRANSFER BUILDING PERMITS WITHOUT THE EXPRESSED WRITTEN COMERKY OF THE STRUCKLESS WRITTEN CONSENT OF THE STRUCTURAL ENGINEER.

CONSENT OF THE STRUCTURAL ENGINEER.

6. CONSTRUCTION BASED ON THE STRUCTURAL DESIGN IS TO BE DONE AS SHOWN IN THE PLANS WITHOUT DEVIATION, CHANGE OR OMISSION WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER. IF ADDITIONAL DETAIL INFORMATION, OR EXPLANATION IS NEEDED, IT IS TO BE OBTAINED FROM THE STRUCTURAL ENGINEER. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR ANY ADDITIONAL PARTS OF THESE PLANS, INCLUDING PROVISIONS AS STATED IN HEM 4.

1. IT IS IMPORTANT TO UNDERSTAND THAT
STRUCTURAL PROVISIONS OF THE BUILDING CODE ARP
COMPLICATED AND THESE PLANS ARE INTENDED TO HE USED BY
AN EXPERIENCED BUILDING CONTRACTOR, PROPERTY OWNERS OBTAINING OWNER-BUILDER PERMITS ARE PROCEEDING AT THEIR OWN RISK. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR ANY ERRORS OR OMISSIONS BY PROPERTY

RESPONSIBLE FOR ANY ERRORS OR OMISSIONS BY PROPERLY OWNERS OR THEIR AGENTS AS A RESULT OF ANY MISUNDERSTANDING OF THE PLANS THE OTHERWISE WOULD BE UNDERSTOOD BY A LICENSED CONTRACTIOR.

8. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, AND SCHEDULE.

9. THE STRUCTURAL PLANS AND ANY RELEVANT DESIGN OWNERS OF THE PROPERTY CHARGE OF THE DOCUMENTS PRODUCED UNDER THE DIRECT CHARGE OF THE STRUCTURAL ENGINEER ARE THE PROPERTY OF THE STRUCTURAL ENGINEER ARE THE PROPERTY OF THE STRUCTURAL ENGINEER AND MAY NOR BE USED BY ANY PERSON OTHER THAN THE CONTRACTED CLIENT AND FOR ANY PURPOSE OTHER THAN THAN THAN STATED IN ITEM 5 ABOVE WITHOUT THE EXPRESSED WRITTEN CONSENT OF THE STRUCTURAL ENGINEER. MOREOVER, NO OTHER ENGINEER OR ARCHITECT IS TO BE DESIGNATED A DIFLEGATED ENGINEER FOR ANY PURPOSE.

DESIGNATED A DIRECTATED INNOINTEER FUR ANY PURPOSE
RELATED TO THESE STRUCTURAL PLANS OR CONSTRUCTION
BASED ON THESE PLANS PRIOR TO THE ISSUANCE OF A
CERTIFICATE OF COMPLETION OR OCCUPANCY WITHOUT THE
EXPRESSED WRITTEN CONSENT OF THE STRUCTURAL ENGINEER.

DESIGN CRITERIA

10. LOAD COMBINATIONS: THIS DESIGN IS BASED ON AN "ALLOWABLE STRESS" FORMULATION RELYING ON THE LOAD COMBINATIONS DEFINED IN FIG. 2017 SECTION 1605.3.1 OR SECTION 1605.3.2 WHERE OMEGA EQUALS 1.3 1). FOUNDATION LOADS: SEE NOTES ON "SITE CONDITIONS.

SOILS, AND FOUNDATIONS*.

SOILS, AND FOUNDATIONS:

12. FLOOR LAVE LOADS:

RESIDENTIAL ONE AND TWO STORY FAMILY DWELLINGS:
ALL LAVE LOADS PER TABLE R301.5

UNINHABITABLE ATTICS WITHOUT STORAGE: 10 PSF

UNINHABITABLE ATTICS WITH STORAGE: 20 PSI

UNINHABITABLE ATTICS AND SUBDING AREAS: 36 PSF HABITABLE ATTICS AND SLEEPING AREAS: 30 PSF BALCONIES, 60 PSF DECKS: 40 PSF ALL OTHER ROOMS 40 PSF GUARDRAILS MANDRAILS: 200PSF CONCENTRATED LOAD APPLIED IN ANY DIRECTION.

13. INFORMATION CONTAINED ON A PLAN SHEET 13. INFORMATION CONTAINED ON A PLAN SHEET WHERE HIS SIGNATURE AND SEAL APPEAR, THAT DOES NOT PERTAIN TO THE RELEVANT STRUCTURAL PROVISIONS AS STATED IN ITEM 4, INCLUDING, BUT NOT LIMITED TO THE BUILDING OCCUPANCY, THE ARCHITECTURAL DESIGN, IT'S FEATURES, FINISHES (LE, DECORATIVE STUCCO, SIDING, ROOFING, SOFFITS, FLASHING, PAINTING, ETC.) AND THEIR INSTALLATION, DIMENSIONS, AND ANY DESIGN OF FIRE PROTECTION, ELECTRICAL, PLUMBING, AND MECHANICAL COMPONENTS OR SYSTEMS. COMPONENTS OR SYSTEMS.

THE ARCHITECTURAL INFORMATION, INCLUDING DIMENSIONS SHOWN IN THESE PLANS AND PROVIDED TO THE ENGINEER. 17. N/A

SITE CONDITIONS

SITE CONDITIONS

18. SITE PLAN AND TOPOGRAPHY
A. THE STRUCTURAL ENGINEER IS NOT A SUVEYOR AND IS
NOT RESPONSIBLE FOR THE SITE PLAN, ESTABLISHING REQUIRED
SET-BACKS, AND LOCATING THE BUILDING ON THE PROPERTY.
B. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR THE
GRADING OF THE SITE OR ITS COMPLIANCE WITH ANY DRAINAGE
PLAN WHETHER INDIVIDUAL OR AS A PART OF A MASTER
DRAINAGE PLAN

PLAN WHE THER INDIVIDUAL OR AS A PART OF A MASTER DRAINAGE PLAN.
C. THE FOUNDATION DESIGN IS BASED ON THESE PRESUMED CONDITIONS INCLUDING THAT DIFFERENTIAL SETTLING IMPESSION OF EXCEED THE SAFE LIMITS OF THE FOUNDATION DESIGN (INCLUDING STEMWALLS AND MASONRY ABOVE GRADE WALLS) AS STATED IN ITEM 19 BELOW.

AS STATED IN TEM 19 BELOW.

D. IT IS IMPORTANT TO KNOW THAT THE FOUNDATION DESIGN
BASED ON A PRESUMED ALLOWABLE SOIL BEARING CAPACITY
OF 2,000 PSF RFLIES ON LESS THAN L/500 (E.G.,0.25 INCHES OVER
10 FBET) OF DIFFERENTIAL SETTLEMENT, CRACKS IN MASONRY
WALLS SHOULD BE EXPECTED WHERE DIFFERENTIAL SETTLEMENT
EXCEEDS L/150.THIS STATEMENT SHOULD BE TAKEN AS A
CAUTIONARY NOTE FOR PROCEEDING WITHOUT A SOILS ANALYSIS
AND FOUNDATION RECOMMENDATION BY A GEOTECHNICAL
ENGINEER FOR THE SITE. ENGINEER FOR THE SITE.

E. COPIES OF ANY AND ALL REQUIRED COMPACTION TESTS ARE TO BE PROVIDED TO THE BUILDING DEPARTMENT FOR THEIR

STRUCTURAL FLEMENTS

19. FOUNDATION, FOOTING AND GROUND FLOOR SLAB
A. THE FOUNDATION AND FOOTINGS ARE TO BEAR A MINIMUM
ON 12 INCHES BELOW GRADE AND ARE TO BE PLACED ON
DISTRICTURED SOIL OF SUL COMPACTED TO A MORE BLOOK UNDISTURBED SOIL OR FILL COMPACTED TO A MINIMUM OF 95% MODIFIED PROCTOR PURSUANT TO ASTM D 1557 WITH FILL LIFTS LESS THAN 12",

COMMERCIAL

ALL LIVE LOADS PER FBC 2017 TABLE 1607.1

14. ROOF LIVE LOADS:

ALL ROOF/WOOD CONSTRUCTION TYPES ARE 30 PSF. 15. DEAD LOADS:

15. DEAD LUAIS:
FLOOR WOOD FRAME: 35 PSF FOR THE MARBLE FLOOR
COVERING, 15 PSF FOR ALL OTHERS.
ROOF WOOD FRAME: 25 PSF FOR SHINGLES, 35 PSF FOR THE

A. WIND LOADS ARE BASED ON THE SPECIFIC REQUIREMENTS AND DEFINITIONS OF FLORIDA RESIDENTIAL BUILDING CODE

2017 EDITION ASCE-7-10. 2017 EDITION ASUS 7-10.

B. THE COMPONENT AND CLADDING WIND PRESSURES ARE
THE MINIMUM REQUIREMENTS FOR STRENGTH AND IMPACT
PROTECTION NEEDED FOR SELECTING SATISFACTORY
COMPONENTS AND CLAUDING, BY OTHERS, FOR THE STRUCTURE.

ENGINEERING BY OTHERS IS PRESUMED ACCURATE AND IS RELIED UPON BY THE STRUCTURAL ENGINEER SOLEY FOR THE PURPOSE OF ACHIEVING COMPLIANCE WITH THE RELEVANT STRUCTURE

20. MIX DESIGNS FOR ALL CONCRETE USED IN THE CONSTRUCTION OF SLAB - ON - GRADE FLOORS SHALL SPECIFY A MINIMUM DESIGN STRENGTH OF 3,000 PSI (20.7 MPa) AT 28 DAYS AND A DESIGN SLUMP NOT TO EXCEED 4 INCHES (102 mm). ON-SITE SLUMPS SHALL NOT EXCEED 5 INCHES (127 mm). PROVIDE TOTAL WATER ADDED TO THE MIX INCLUDING PLANT, TRANSIT AND SITE ADDED WATER DOES NOT EXCEED THE EXALLOWING BARAGETERS. EXCERD THE FOLLOWING PARAMETERS:

1. FOR MIXES USING NATURAL SANDS: 275 POUNDS PER CUBIC YARD

(33 GALLONS-125L) 2. FOR MIXES USING MANUFACTURED SANDS: 292 POUNDS PER CUBIC YARD (35 OALLONS -132L.)

A. IN ADDITION, THE STRUCTURAL ENGINEER IS NOT A CIVIL OR GEOTECHNICAL ENGINEER AND IS NOT RESPONSIBLE FOR DETERMINING THE SUITABILITY OF THE SITE FOR CONSTRUCTION, GEOTECHNICAL ENGINEER AND IS NOT RESPONSIBLE FOR DETERMINING THE SUITABILITY OF THE SITE FOR CONSTRUCTION, INCLUDING ITS TOPOGRAPHY, DRAINAGE AND SUB-SURFACE CONDITIONS (INCLUDING WATER TABLE DEPTH) AND FOR INTERPRETING GEOTECHNICAL DATA CONCERNING THE SITE. B. IF SOIL CONDITIONS AT THE SITE APPEAR QUESTIONABLE AS DETERMINED BY THE BUILDING CONTRACTOR OR OWNER-HUILDER, A SOILS ANALYSIS SHALL BE PERFORMED BY A LICENSED GEOTECHNICAL ENGINEER THAT WILL GIVE SPECIFIC RECOMMENDATIONS FOR A FOUNDATION TYPE IF THE BUILDING CONTRACTOR OR OWNER-BUILDER DO NOT MAKE THAT DETERMINATION AND A SOILS ANALYSIS IS NOT PERFORMED, THE STRUCTURAL ENGINEER SHALL PROCEED WITH THE DESIGN BASED ON THE PRESUMPTIONS ALLOWED BY THE FBC 2017, SEC. 1804. C. THE DETERMINATIONS OF THE SUITABILITY OF THE SITE FOR CONSTRUCTION (INCLUDING TOPOGRAPHICAL INFORMATION) AND THE SOIL CONDITIONS SHALL HAVE BEEN COMPLETED AND ANY RECOMMENDATIONS RESULTING FROM THAT ANALYSIS SHALL BAVE BEEN PROVIDED TO THE STRUCTURAL ENGINEER PRIOR TO THE SIGNING AND SEALING OF THE STRUCTURAL PLANS.

D. IN THE ABSENCE OF GEOTECHNICAL INFORMATION, THE SITE IS PRESUMED TO HAVE AN ALLOWABLE E SOIL BEARING CAPACITY OF 2000 PSF AND THE TOPOGRAPHY AS IT RELATES TO THE STRUCTURE IS PRESUMED TO BE THAT SHOWN IN THE PLANS.

E. THE SIZE AND REQUIRED REINFORCEMENT FOR THE FOOTINGS ARE SHOWN ON THE FOUNDATION PLAN.

HE GROUND FLOOR SLAB SHALL BE PLACED OVER A 6 MIL.

POLYETHYLENE MOISTURE RETARDER.

I. THE TRUSS SYSTEM DESIGN PROVIDED IN THIS PLAN IS FOR THE USE OF THE TRUSS MANUFACTURER IN DEVELOPING THE ACTUAL ROOF TRUSS SYSTEM DESIGN. IT IS NOT TO BE USED FOR ANY OTHER PURPOSE AS IT IS SUBJECT TO ENGINEERING AND MAY BE DIFFERENT FROM THE FINAL DESIGN. II. MANUFACTURED FLOOR TRUSSES SHALL BE DESIGNED BY A LICENSED TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER ACTING AS A DELEGATED ENGINEER AND WORKING THROUGH A TRUSS MANUFACTURER FOR THIS PURPOSE. THE SELECTION OF THE TRUSS MANUFACTURER IS HEREBY SUBORDINATED TO THE BUILDING CONTRACTOR.

BUILDING CONTRACTOR.

III. THE MANUFACTURED TRUSS DESIGN SHALL INCLUDE SPECIFYING THE TRUSS TO TRUSS AND TRUSS TO GIRDER CONNECTIONS ON EITHER THE INDIVIDUAL TRUSS COMPONENT SHEETS OR THE GIRDER TRUSS COMPONENTS SHEETS AS APPLICABLE A SPECIFIC HANGER MUST BE SELECTED AND IDENTIFIED ON THE SIGNED AND SPALED COMPONENT SHEETS FOR EACH LOCATION THAT A HANGER IS REQUIRED IN THE

IRUSS SYSTEM.

IV. THE TRUSS PLAN SIGNED AND SEALED BY THE DELEGATED ENGINEER SHALL BE PROVIDED TO AND REVIEWED BY THE STRUCTURAL ENGINEER FOR COMPLYING WITH THE DESIGN INTENT OF THE ORIGINAL PLAN AND FOR ANY CHANGES TO THE STRUCK TO COMPUTIONS. THE TREES TO UNDERLYING STRUCTURE CONNECTIONS.
THIS PLAN MUST BE PROVIDED TO THE STRUCTURAL ENGINEER
PRIOR TO CONSTRUCTION ON THE UNDERLYING STRUCTURE AS
THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO MAKE
STRUCTURAL CHANGES BASED UPON THE FINAL FLOOR TRUSS
GYSTIM

P. CONVENTIONAL FRAMED JOISTS WITH A MINIMUM 6 INCH OVERLAP OF JOINTS. G. TERMITE TREATMENT OF THE SITE SHALL BE SPECIFIED BY THE BUILDING CONTRACTOR OR OWNER-BUILDER. H. SHRINKAGE CONTROL OF THE FLOOR SLAB SHALL BE ACCOMMUNICATION OF THE FLOOR SLAB SHALL BE H. SHRINKAGE CONTROL OF THE FLOOR SLAB SHALL BE ACCOMPLISHED BY 6 INCH BY 6 INCH W 1.4 BY 1.4 WE DED WRE FABRIC AS SPECIFIED BY FBC 2017 SECTION 1910.2 EXCEPTION 2 OR FIBERMESH ADMIXTURE AS SPECIFIED BY FBC 2017, SECTION 1910.2 EXCEPTION 1. THE WELDED WIRE FBC 2017, SECTION 1910.2 EXCEPTION 1. THE WILDED WIRE FBC 2017, SECTION 1910.2 EXCEPTION 1. THE WILDED WIRE FBC 2017, SECTION 1910.2 EXCEPTION 1. THE WILDED WIRE FACE AND UPPER 1.3 DEPTH OF THE STAR AND UPPER 1.3 DEPTH OF FABRIC SHALL BE PLACED BETWEEN THE MIDDLE AND UPPER 13 DEPTH OF THE SLAB AND HELD IN POSITION BY APPROPLATE SUPPORTS SPACED NOT GREATER THAN 3 FIET APART, L. CONTRACTION JOINTS ARE TO BE PROVIDED FOR THE PURPOSE OF CONTROLLING SHRINKAGE.ONE INCH DEEP CUTS (FOR A FOUR INCH THICK SLAB OR 25 PERCENT OF THE SLAB THICKNESS OTHERWISE) ARE TO BE PROVIDED ACROSS THE WIDTH AND LENGTH OF ANY FLOOR SLAB AT A DISTANCE OF NOT TO EXCEED 30 TIMES THE SLAB THICKNESS. FOR EXAMPLE A FOUR INCH THICK SLAB. CONTRACTION JOINTS SHALL NOT A FOUR INCH THICK SLAB, CONTRACTION JOINTS SHALL NOT A FOURTNESS THICK STAD, CONTRACTION JOINTS SHALL NO EXCEED IN FIFET ON CENTIFIC EACH WAY, THE CONTRACTION JOINTS ARE OPTIONAL FOR ONE AND TWO STORY PAMILY RESIDENTIAL WHEN WELDED WIRE FABRIC OR FIBERMESH ARE USED IN THE FLOOR SLAB.

STRUCTURAL ENGINEER NOTES

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MODEL

COVE CONCORD DR. DUNEDIN LOT

DATE

DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, IL 194557 727-376-6831

A. MANUFACTURED FLOOR TRUSS FRAMING PLAN CONTAINED HEREIN IS FOR THE SOLE PURPOSE OF

- ELLUSTRATING THE DESIGN INTENT AND FOR PLANNING TO BE USED BY THE TRUSS COMPANY.

 1. FLOOR JOISTS ARE SIZED BASED ON THE SOUTHERN PINE COUNCIL SPAN TABLES FOR NO. 2 GRADE DIMENSIONAL LUMBER.
- 11. FLOOR JOISTS FOR EXTERIOR DECKS SHALL BE PRESSURE TREATED.
- B. FOR ALL WOOD FLOORS:
- 1. THE TRUSS TO WALL CONNECTIONS ARE IDENTIFIED
- I. THE TRUSS TO WALL CONNECTIONS ARE IDENTIFIED ON THE FLOOR FRAMING PLAN.

 B. A STRUCTURAL BAND JOIST IS TO BE PROVIDED ON THE EXTERIOR PERIMETER OF ALL BUTTOM BEARING FLOOR TRUSSES AND JOISTS THE STRUCTURAL BAND JOIST IS TO BE PASTENED TO EACH END OF A FLOOR TRUSS OR JOIST WITH A SIMPSON LSD BRACKET USING CHAPSON SUPPLY TO CAMBON MADES. SIMPSON SHORT TO COMMON HAILS.
- III. FLOOR TRUSSES OR JOISTS BEARING ON WOOD WALLS ARE TO BE SET WITH A MINIMUM OF THREE 104 COMMON
- NAILS (TOE NAILED) TO THE TOP PLATE OF THE WALL.

 IV. A MOISTURE BARRIER SHALL BE INSTALLED BETWEEN ANY
 UNTREATED WOOD TRUSSES OR JOISTS AND CONCRETE OR ANY MASONRY.
- V. LEDGERS NAILERS SHALL BE FASTENED TO WOOD STUDS OR BAND JOISTS (NOT SHEATHING) WITH A MINIMUM 2 38" X 5 P2" LAG BOLTS WITH WASHERS AT FACH STUD INTERSECTION AT 16 INCHES ON CENTER AND SHALL CONSIST OF PRESSURE TREATED I UMBER 2 PLY 1 1/2" THICK BY A HEIGHT SHOWN IN THE PLANS. FOR CONCRETE OR MASONRY WALLS THE FASTENERS SHALL BE 5/8" X 5 1/2" SIMPSON TITEN HEAD CONCRETE BOLTS.

- BEAMS SUPPORTING FLOOR TRUSSES AND JOISTS ARE TO BE ATTACHED AS SPECIFIED IN THE FLOOR FRAMING PLAN. UNDER NO CIRCUMSTANCES ARE THERE TO BE BHITT JOINTS BETWEEN THE BEARING POINTS OF ANY PLY OF A MULTIPLE BEAM. THE PLIES ARE TO BE CONTINUOUS BETWEEN BEARING POINTS.
- BETWEEN BEARING POINTS.
 MULTIPLE BEAMS CONSISTING OF MANUPACTURED WOOD
 (I.E. GLULAM, MICROLAM) ARE YO HAVE THE INDIVIDUAL
 PLIES INTERCONNECTED AS REQUIRED BY THE MANUFACTURERS
- 4. MULTIPLE BEAMS CONSISTING OF DIMENSIONAL LUMBER ARE TO HAVE INDIVIDUAL PLIES INTERCONNECTED AS FOLLOWS: A FOR TWO PLY BEAMS- ONE ROW OF ION GALVANIZED COMMON
- NAILS AT 6"O.C. ON EACH SIDE OF THE BEAM

 II. FOR THREE PLY BEAMS. TWO ROWS OF 164 OALVANIZED
 COMMON NAILS SPACED AT 6"O.C. (TOP AND BOTTOM) THRU
- C. FOR FOUR PLY BEAMS OR LARGER-TWO ROWS OF 1/2" DIAMETER CARRIAGE BOLTS OR ALL THREAD ROD WITH NUTS AND WASHERS SPACED AT 12 INCHES ON CENTER, 2 INCHES FROM THE TOP AND BOTTOM EDGES OF THE BEAM.
- D. FLOOR SHEATHING:

 1. ALL FLOOR SHEATHING IS TO BE 3/4" TONGUE AND GROOVE PLYWOOD RATED FOR FLOOR SHEATHING
- II. FLOOR SHEATHING SHALL BE FASTENED TO THE FLOOR TRUSSES JOISTS WITH 103 RING SHANK NAILS AT 6" ON CENTER WITH CONSTRUCTION GRADE ADHESIVE.
- III. FLOOR SHEATHING SPECIFIED FOR SEALED EXTERIOR DECKS AND ITS INSTALLATION SHALL BE THE SAME AS THAT FOR INTERIOR APPLICATION EXCEPT PRESSURE FREATED AND THE FASTENERS TO BE GALVANIZED. E. EXTERIOR DECK FLOORING:
- DECK FLOORING SHALL BE INDIVIDUALLY SPECIFIED ON THE FLOOR FRAMENG PLANS AND SHALL BE PASTENED TO THE UNDERLYING PRESSURE TREATED JOISTS WITH 3.
 3 INCH DECK SCREWS AE EACH FLOORING JOIST INTERSECTION.

22. WALLS:

- A. MASONRY

 I. CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI.

 II. WALL CMU SHALL BE 8 INCH X 16 INCH IN SIZE OR 8 INCH X 8 INCH X 8 INCH FOR EDGE FINISHES.

 III. CMU SHALL BE PLACED IN A RUNNING BOND AND THERE SHALL BE NO VERTICAL BUTT JOINTS EXCEPT AS SHOWN ON THE FLOOR PLAN FOR CONSTRUCTION JOINTS.

 IV. REINFORCED FILLED CELLS AS SHOWN ON THE PLANS SHALL BE FILLED WITH "FINE" GRADE GROUT, HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1,000 PSI AND 8 TO 11 INCH SLUMP TO ENSURE CONSOLIDATION.

 V. BOND BEAMS SHALL BE POURED WITH GROUT MOYOU THERE SALE.
- IO ENSURE CONSOL DATION.

 V. BOND BEAMS SHALL BE POURED WITH GROUT MONOLITHICALLY WITH THE FILLED WALL CELLS-NO COLD JOINTS.

 VL VERTICAL STEEL REINFORCEMENT SHALL BE CONTINUOUS BETWEEN THE MIDDLE AND BOTTOM 1/3 OF THE FOOTING HEIGHT AND END IN THE TOP COURSE OF THE BOND BEAM WITH A STANDARD IN INCH ON DEED FEER D. IO INCH 90 DEGREE BEND,
- VII. HORIZONTAL REINFORCING STEEL SHALL BE CONTINUOUS, INCLUDING
- VIII. REINFORCING STEEL SPLICES SHALL CONSIST OF WIRE LAPS NO LESS THAN 40 TIMES THE STEEL BAR DIAMETER (LE. 25 INCHES FOR #5 REBAR, 15 INCHES FOR #3 REBAR, AND 52 INCHES FOR #7 REBAR)
- B. WOOD FAME WALLS:
- 1. WALL STUD SIZES ARE SHOWN IN THE TYPICAL WALL SECTION.
- II. LOAD BEARING.

 I. WOOD STUDS IN WALLS SHALL BE SPACED IS INCHES ON CENTER AND FASTERIED TO THE TOP AND BOTTOM PLATES PER THE TOP PLATE SPLICE BETAIL. ALL LOAD BEARING STUDS TO BE SOUTHERN YELLOW PINE #2
- GRADE OR BETTER.
 2. LOAD BEARING WALLS SHALL HAVE A SINGLE BOTTOM PLATE (PRESSURE TREATED) IN CONTACT WITH MASONRY OR CONCRETE, SEE THE TOP PLATE SPICE DETAIL FOR TOP PLATE NAILING AND
- SPLICING REQUIREMENTS.

 3. THE WOOD STUDS SHALL HAVE A SIMPSON SP2 AT THE TOP PLATE AND A PROPERLY SIZED SPH FOR THE BOTTOM PLATE (L.B. 4" STUD WALL = SPH4,
- 4. 3 STUD PACK SHALL BE INSTALLED DIRECTLY BENEATH BEARING POINTS
 OF ALL GIRDERS AND BEAMS HAVING A GRAVITY LOAD OF UP TO 3,000 LBS.
 5. STEEL TUBE COLUMNS SHALL BE INSTALLED IN THE WALL DIRECTLY HENEATH
- 5. STIEL TUBE COLUMNS SHALL BE INSTALLED IN THE WALL DIRECTLY BENEGORDERS AND BEAMS HAVING GRAVITY LOADS GREATER THAN 3000 LBS.

 6. BASE PLATES SHALL BE FASTENED TO MONOLITHIC FOOTINGS WITH 58° X 8 INCH ANCHOR BOLTS OR SIMPSON TITEN HD. CONCRETE BOLTS OF THE SAME SIZE AT 24 INCHES ON CENTER. ALL CONNECTIONS SHALL BE MADE WITH 3 INCH SQUARE BY 1/8 INCH THICK WASHERS.

 7. BASE PLATES BEARING ON WOOD SHALL BE FASTENED WITH 164 COMMON NAILS AT 8° O.C. THROUGH ANY FLOOR SHEATHING AND TO UNDERLYING LUMBER (NOT SHEATHING ONLY) AND USE BLOCKING AS NEEDED TO MAINTAIN MAILING SPACING RECHIRPMENTS.

- LUMBER (NOT SHEATHING ONLY) AND USE BLOCKING AS NEEDED TO MAINTAIN MAILING SPACING REQUIREMENTS.

 8. FOR EXTERIOR LOAD BEARING WALLS, EACH STUD ABOVE THE BASE PLATE SHALL BE FASTENED TO THE UNDERLYING BAND JOIST OR BEAM WITH A SIMPSON LSTAIR STRAPFOR THIS SITUATION THE SIMPSON SPH BRACKET TO THE BASE PLAN MAY BE OMITTED.

 9. FOR INTERIOR LOAD BEARING WALLS, 1/2 INCH ALL THREAD ROD SHALL BE INSTALLED AT 32° O.C. FROM THE BASE PLATE THROUGH THE SHEATHING AND TOP PLATE OF UNDERLYING SUPPORTING WALL, ALL CONNECTIONS SHALL INCLUDE A STANDARD 3 INCH SOHARE WASHER.
- AND 10P PLATE OF UNDERLYING SUPPORTING WALL, ALL CONNECTIONS SHALL INCLUDE A STANDARD 3 INCH SQUARE WASHER.

 10. HEADER BEAMS SHALL BE SIZED ACCORDING TO THE ENCLOSED HEADER SCHEDULE AND PASTENED WITH A MINIMUM OF TWO SIMPSON LSTA36 STRAPS OVER EACH END TO THE JACK STUDS BELOW, IN ADDITION, THE HEADER BEAMS SHALL BE FASTENED WITH A MINIMUM OF 3-10d COMMON NAILS (TOE NAILED ON EACH PACE SIDE AT EACH END TO THE ABUTTING FILL LENGTH CTUTOS. FULL LENOTH STUDS.
- III. NON LOAD BEARING WALLS:

 1. WOOD STUDS IN WALLS SHALL BE SPACED AT 16 INCHES ON CENTER AND FASTENED TO THE TOP AND BOTTOM PLATES WITH A MINIMUM OF THREE 104 COMMON NAILS, NAILS INSTALLED IN PRESSURE TREATED WOOD SHALL BE GALVANIZED.
- INCIDENTAL, NON STRUCTURAL FRAMING ITEMS SUCH AS KNEE WALLS, DROP CEILINGS, BUILT IN SHELVING, NICHES, ETC. MAY BE CONSTRUCTED WITH 2 X 4'S AT 24' O.C. AT THE DISCRETION OF THE BUILDER.

- 2. NON LOAD BEARING WALLS SHALL HAVE A SINGLE BOTTOM PLATE (PRESSURE TREATED AGAINST MASONRY AND CONCRETE) AND A SINGLE TOP PLATE.
- 3. BASE PLATES SHALL BY FASTENED TO CONCRETE SLAUS WITH 1/4 INCH BY 3 1/2 INCH TAPCON SCREWS AT 12 "ON
- 4. BASE PLATES ON WOOD SHALL BE FASTENED WITH 166 COMMON NAILS AT 8" ON CENTER.

- C. SHEATHING
 I. PLYWOOD SHEATHING.

 1. EXTERIOR WALL SHEATHING COVERED BY AN ARCHITECTURA),
 FINISH SHALL BE MINIMUM 7/16 INCH THICK (NOMINAL) 4 PLY
 PLYWOOD MANUFACTURED WITH EXTERIOR GLUE,
 2. THE LONG SIDE OF THE SHEATHING SHALL BE INSTALLED
 PERPENDICULAR TO THE WALL STUDS.
 3. FASTEN TO STUDS AND BLOCKING WITH BURING SHANK NAILS
 AT 4 INCHES ON CENTER ALL LOCATIONS.
 4. IN ADDITION TO THE REGULAR FASTENING A SECOND ROW SHALL
 BE INSTALLED AT THE DOUBLE TOP PLATE AND TO THE LOWEST
 HORIZONTAL WOOD MEMBER ON AN EXTERIOR WALL.
 (1 E. SILL PLATE, BAND JOIST)
- HORIZON FAL WOOD MERIDER OF AN EATERSAN FORM:
 (1E SILL PLATE, BAND JOIST)
 5. FOR PLYWOOD SHEATHING COVERED WITH A CEMENTITIOUS FINISH ALL BUILT JOINTS NOT ON WALL STUDS SHALL BE BLOCKED WITH 2 X BLOCKING, TOE NAILED AT EACH END TO THE WALL STUDS WITH 3-84 COMMON NAILS.
- JL PARTICLE BOARD
- I. PARTICLE BOARD IS NOT TO BE USED WITHOUT THE EXPRESS, WRITTEN CONSENT OF THE STRUCTURAL ENGINEER AND THE PROPERTY OWNER.
- PROPERTY OWNER.
 III. ARCHITECTURAL FINISHES
 L ARCHITECTURAL WALL FINISHES, SUCH AS STUCCO, CEMENTITIOUS COATING, SIDING OR PAINT ARE MENTIONED HERE ONLY FOR THE PURPOSE OF UNDERSTANDING THAT THEIR INSTALLATION AND ASSOCIATED DETAILS ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

- 23. COLUMINS
 A. CONCRETE / MASONRY COLUMNS
 1. MASONRY COLUMNS SHALL HE CONSTRUCTED OF PILASTER CONCRETE BLOCK OR FORMED AND POURED, WALL BLOCK SHALL NOT RE USED FOR MASONRY COLUMNS.
- FOR MASURKY COLUMNS.

 II. REINFORCING STEEL SHALL BE GRADE 60 AND HELD IN PLACE BY STIRUPS SPACED AT 12 INCHES ON CENTER VERTICALLY.

 III. PLASTER BLOCK COLUMNS SHALL BE FILLED WITH A FINE GROUT
- HAVING A MINIMUM OF COMPRESSIVE STRENGTH OF 3,000 PSI

 IV. FORMED AND POURED COLLAMNS SHALL CONSIST OF A MINIMUM
 OF 3,000 PSI CONCRETE, OR IN AREAS OF HIGH CHLORIDES, SUCH
 AS NEAR THE COAST OR BODIES OF SALT WATER, THE MINIMUM
- SHALL BE 5,000 PSI
- V. ALI MASONRY COLUMNS SHALL BEGIN AT THE FOUNDATION OR AT A MONOLITHIC FOOTING, IN NO CASE SHALL THERE BE A BREAK OR A COLD JOINT IN THE GROUT OF A COLUMN EXCEPT AT 1 FOOT FROM THE TOP IN PREPARATION FOR INSTALLATION OF A CONCRETE LINTEL.
- THE COP IN PREPARATION FOR INSTALLATION OF A CONCRETE LINTEL.

 VL METAL CONNECTORS AT THE TOP OF THE COLUMN FOR BOLDINO WOOD BEAMS OR GIRDERS SHALL BE INSTALLED WITH THE MINIMUM EMBEDMENT OF THE ASSOCIATED FASTENERS FOR THE CONNECTOR AS SHOWN ON THE PLANS.
- B. WOOD COLUMNS:
- ALL LOAD BEARING WOOD COLUMNS SHALL BE A MINIMUM OF #2 GRADE PRESSURE TREATED WOOD,
- II. DIMENSIONAL WOOD COLUMNS OF 4 INCHES BY 4 INCHES IN CROSS SECTION SHALL ONLY BE USED FOR SUPPORTING OPEN WOOD DECKS WHERE THE FLOOR HPIGHT ABOVE THE FLOOR BELOW IS 8 FEET OR LESS.
 ALL OTHER DIMENSIONAL WOOD COLUMNS SHALL HAVE A MINIMUM OF 6 INCHES BY 6 INCHES.
- 6 INCHES BY 6 INCHES.

 III. METAL CONNECTORS AT THE BASE AND THE TOP OF WOOD COLUMNS SHALL BE OF THE TYPE THAT RESISTS LATERAL LOADS AS WELL AS UPLIFT AND GRAVITY LOADS, IN NO CASE SHALL FLAT STRAPS BE USED UNLESS SPECIFICALLY SHOWN IN THE PLANS OR CROSS SECTION DETAILS.

NOTE RUCTURAL ENGINEER

MODEL

ALLEN ENCONSTRUC

DUNEDIN COV CONCORD DR. LOI

DEEB FAMILY HOMES, LTD.
SEW PORT RICHEY, EL 34655
727-376-6851

C. COMPOSITE COLUMNS

- COMPOSITE COLUMNS
 A COMPOSITE COLUMN HERE IS DEFINED AS A HOLLOW COLUMN
 CONSISTING OF ANY MATERIAL SPECIFICALLY DESIGNED BY ITS
 MANUFACTURER TO BE LOAD BEARING, ANY OTHER TYPE OF HOLLOW COLUMN IS CONSIDERED AN ARCHITECTURAL FINISH INTENDED TO FIT OVER A STRUCTURAL COLUMN AND ITS USE AND DETAILS OF INSTALLATION ARE NOT THE RESPONSIBILITY OF THE
- DETAILS OF INSTALLATION ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

 II. LOAD BEARING COMPOSITE COLUMNS ARE A MANUFACTURED PRODUCT SUBJECT TO THE DESIGN AND LOAD BEARING CAPACITY AS DETERMINED BY THE MANUFACTURER. A SHOP DRAWING OR A LETTER FOR THE INSTALLATION OF THE COLUMN SHALL BE PROVIDED BY THE STRUCTURAL ENGINEER TO SUPPLEMENT THE CONSTRUCTION PLANS AFTER THE SPECIFIC COLUMN AND ACTURED BY THE SPECIFIC COLUMN AND AC COLUMN AND MANUFACTURER HAVE BEEN IDENTIFIED.
- HEIN ALL CASES. THE COLUMN MANUFACTURES INFORMATION SHALL BE PROVIDED TO THE STRUCTURAL ENGINEER BY THE CONTRACTING CLIENT OR HIS AGENT FOR REVIEW PRIOR TO ITS ACCEPTANCE FOR THE STRUCTURAL DESIGN. THE INFORMATION SHALL INCLUDE THE LATERAL AS WELL AS UPLIFT AND GRAVITY LOAD BEARING CAPACITIES.

D., STEEL TUBE COLUMNS:

- L. LOAD BEARING STEEL TUBE COLUMNS SHALL HAVE A MINIMUM WALL THICKNESS OF 1/4 INCH AND BE MADE OF STEEL WITH A DESIGN YIFLD STRENGTH OF 46 PSI UNLESS OTHERWISE SHOWN IN THE STRUCTURAL DESIGN
- II. THE SPECIFIC CONNECTION SCHEME SHALL BE SHOWN IN THE STRUCTURAL DESIGN WHERE THE STEEL TUBE COLUMN IS TO BE INSTALLED.

F. ALUMINUM COLUMNS:

- 1. LOAD BEARING ALUMINUM COLUMNS SHALL HAVE A MINIMUM WALL THICKNESS
- II. ALL FASTENERS AND CONNECTORS FOR ALUMINUM COLUMNS SHALL BE STAINLESS STEEL OR MONEL TO AVOID CORROSION DUE TO DISSIMILAR METALS BEING IN CONTACT.
- III. THE SPECIFIC CONNECTION SCHEME SHALL BE SHOWN IN THE STRUCTURAL DESIGN WHERE THE ALLOMINUM COLUMN IS TO BE INSTALLED.

A. MANUFACTURED WOOD TRUSSES

- 1. THE MANUFACTURED ROOF TRUSS FRAMING PLAN CONTAINED HEREIN IS FOR THE SOLE PURPOSE OF ILLUSTRATING THE DESIGN INTENT AND FOR PLANNING TO BE USED BY THE TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER OF THE TRUSS MANUFACTURER IN DEVELOPING THE ACTUAL SYSTEM DESIGN, IT IS NOT INTENDED TO BE USED FOR ANY OTHER PURPOSE AS IT IS SUBJECT TO ENGINEERING AND MAY BE DIFFERENT FROM THE FINAL
- II. MANUFACTURED ROOF TRUSSES SHALL HE DESIGNED BY A LICENSED TRUSS COMPONENT AND TRUSS SYSTEM ENGINEER ACTING AS A DELEGATED ENGINEER AND WORKING THROUGH A TRUSS MANUFACTURER FOR THIS PURPOSE. THE SELECTION OF THE TRUSS MANUFACTURER IS HEREBY
- PURPOSE. THE SELECTION OF THE TRUSS MANUFACTURER IS HEREBY SUBORDINATED TO THE BUILDING CONTRACTOR.

 III. THE TRUSS PLAN "SIGNED AND SEALED" BY THE DELEGATED ENGINEER SHALL BE PROVIDED TO AND PRIOR TO CONSTRUCTION OF THE UNDERLYING STRUCTURE AS THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO MAKE STRUCTURAL CHANGES BASED ON THE FINAL FLOOR TRUSS SYSTEM.

 VI. THE TRUSS MANUFACTURER SHALL PROVIDE ALL LATERAL BRACING REQUIREMENTS TO THE BUILDING CONTRACTOR IS TO NOTIFY THE STRUCTURAL ENGINEER FOR GUIDANCE.

 V. IN ADDITION TO THE METAL CONNECTORS SHOWN IN THE TRUSS LAYOUT OF THE ORIGINAL PLANS, EACH TRUSS IS TO BE SET ON WOOD FRAME BEARING WALLS OR SILL PLATES WITH 104 COMMON NAILS (TOE-NAILED)

 VI. A MOISTURE BARRIER IS TO BE INSTALLED BETWEEN UNTREATED WOOD AND CONCRETE/MASONRY

23.2 CONVENTIONAL FRAME

- 1. IN ADDITION TO THE METAL CONNECTORS SHOWN IN THE TRUSS LAYOUT OF THE ORIGINAL PLANS, EACH RAFTER IS TO BE SET ON WOOD FRAME BEARING WALLS OR SHA PLATES WITH 3- 10d COMMON NAILS (TOE-NAILED)
- II. ANY WOOD COMING IN CONTACT WITH MASONRY OR CONCRETE IS TO BE PRESSURE TREATED OR A MOISTURE BARRIER IS TO BE INSTALLED BETWEEN UNITERATED WOOD AND CONCRETE OR MASONRY.

181. COLLAR TIES ARE TO BE INSTALLED BETWEEN RAFTERS AT 2/3 OF THE RIDGE HEIGHT FROM WHERE THE RAFTERS BEAR ON WALLS. THE COLLAR TIES ARE TO BE FASTENED WITH A MINIMUM OF 4-100. 16 COMMON NAILS (CLINCHED) AT EACH LAP JOINT, EACH RAFTER IS TO BE ATTACHED TO THE RIDGE BEAM WITH A LIGHT ANGLE HANGER AS SHOWN IN THE FRAMINED FAN IN ADDITION A ELAT METAL STEAM SHALL IN FRAMING PLAN. IN ADDITION, A FLAT METAL STRAP SHALL BE INSTALLED ACROSS THE RIDGE BEAM TO TWO OPPOSING RAPTER. TO BE REVIEWED BY THE STRUCTURAL ENGINEER FOR COMPLYING WITH THE DESIGN INTENT OF THE ORIGINAL PLAN AND FOR ANY CHANGES TO THE *TRUSS TO THE UNDERLYING STRUCTURE* CONNECTIONS.

IV. AS PART OF THE REVIEW, THE STRUCTURAL ENGINEER WILL DETERMINE WHETHER THE TRUSS TO WALL / BEAM METAL CONNECTORS SHOWN IN THE ORIGINAL PLANS ARE ACCEPTABLE OR WHETHER THEY NEED TO BE CHANGED OR SUPPLEMENTED TO ACCOMMODATE THE LOADS SHOWN IN THE TRUSS COMPONENT

VI THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR VERIFYING THE DIMENSIONAL, ARCHITECTURAL, OR FORM ASPECTS OF THE OF THE TRUSS MANUFACTURERS PLAN WITH THE ORDINAL PLANS. VI. THE MINIMUM LIVE LOADS FOR THE ROOF TRUSS DESIGN IS TO BE

ON FBC 2017 SECTION 1607 FOR ROOF TYPE AND ROOFING MATERIAL VIL THE DEAD LOADS ARE LASTED IN ITEM 16 ABOVE.

VII. THE DEAD LOADS ARE LASTED IN 1TEM 16 ABOVE.

VIII. ALL TRUSS TO TRUSS AND TRUSS TO GIRDER CONNECTORS ARE TO BE SPECIFIED BY THE TRUSS MANUFACTURER, INCLUDING CONNECTORS FOR TRUSS TO MANUFACTURED BEAM (I.E. GLUELAM, OR MICROLAM) SPECIFIED BY THE TRUSS MANUFACTURER, A SPECIFIC HANGER MUST BE SELECTED AND IDENTIFIED ON THE SIUNED AND SEAL BD COMPONENT SHEETS FOR FACH LOCATION, A HANGER IS BEGOVERD IN THE TURGS SVEITEM. A HANGER IS REQUIRED IN THE TRUSS SYSTEM.

A HANGER IS REQUIRED IN THE TRUSS SYSTEM.

IX. THE TRUSS PLAN SIGNED AND SEALED BY THE DELEGATED ENGINEER SHALL BE PROVIDED TO AND REVIEWED BY THE STRUCTURAL ENGINEER FOR COMPLYING WITH THE DESIGN INTENT OF THE ORGINALPLAN AND FOR ANY CHANGES TO THE TRUSS TO UNDERLYING STRUCTURE CONNECTIONS. THIS PLAN MUST BE PROVIDED TO THE STRUCTURAL ENGINEER.

X. A RIDGE BEAM TERMINATING AT A GABLE END SHALL BE SUPPORTED BY A MINIMUM 3 STUD PACK COLUMN BEARING ON THE UNDERLYING

WALL OR BEASI.

XI. TREATED LUMBER-IXOUBLE 1 1/2 INCH BY A HEIGHT SHOWN ON THE PLANS, FOR CONCRETE OR MASONRY WALLS THE FASTENERS SHALL. BE 5/8 INCH BY 5 1/2 INCH SIMPSON TITEN HD CONCRETE BOLTS.

XII. SLEEPERS SHALL BE FASTENED TO UNDERLYING ROOF TRUSSES

OR RAFTERS (NOT SHEATHING) WITH A MINIMUM OF 2-38 INCH BY 3 1/2 INCH LAG BOLTS AND WASHERS AT EACH TRUSS OR RAFTER INTERSECTION AND NO GREATER THAN 24 INCHES ON CENTER AND SHALL CONSIST OF DIMENSIONAL LUMBER 1 1/2 INCH THICK

BY A WIDTH SHOWN IN THE PLANS.

XIII. USE 2 INCH BY 4 INCH BLOCKING ATTACHED BETWEEN UNDERLYING STUDS, TRUSSES OR RAFTERS WITH A MINIMUM OF 3-104 NAILS AT EACH IN ORDER TO SATISFY THE ON CENTER SPACING FOR THE LEDGERS OR SLEEPERS.

XIV BEAMS SUPPORTING ROOF TRUSSES OR RAFTERS ARE TO BE ATTACHED AS SPECIFIED IN THE ROOF FRAMING PLANS.

24. UNDER NO CIRCUMSTANCES ARE THERE TO BE BUTT JOINTS BETWEEN THE BEARING POINTS OF ANY PLY OF A MULTIPLE BEAM. THE PLIES ARE TO BE CONTINUOUS BETWEEN BEARING POINTS.

LEDGERS/ SLEEPERS

LEDGERS / NAILERS SHALL BE FASTENED TO WOOD STUDS (NOT SHEATBING) WITH A MINIMOM OF 2-38 INCH BY 5-1/2 INCH LAG BOLTS WITH WASHERS AT EACH STUD INTERSECTION AND NO GREATER THAN 16 INCHES ON CENTER AND SHALL CONSIST ON PRESSURE TREATED WOOD.

MULTIPLE BEAMS CONSISTING OF MANUFACTURED WOOD (LE. GLUBLAM, MICROLAM) ARE TO HAVE THE INDIVIDUAL PLIES INTERCONNECTED AS REQUIRED BY THE MANUFACTURERS SPECIFICATIONS.

BL MULTIPLE BEAMS CONSISTING OF DIMENSIONAL LUMBER ARE TO HAVE THE INDIVIDUAL PLIES INTERCONNECTED

I. FOR TWO PLY BEAMS - ONE ROW OF IOI GIALVANIZED COMMON NAILS AT 6 INCHES ON CENTER ON EACH SIDE OF BEAM.

II. FOR THREE PLY BEAMS - TWO ROWS OF I64 GIALVANIZED COMMON NAILS AT 6 ON CENTER (TOP AND BOTTOM)

THRU EACH SIDE OF THE BEAM.

III.FOR FOUR PLY BEAMS AND LARGER-TWO ROWS OF 1/2 INCH
DIAMETER CARRIAGE BOLTS OR ALL THREAD RODS WITH NUTS
AND WASHERS SPACED AT 12" ON CENTER 2 INCHES FROM THE
TOP AND BOTTOM EDGES OF THE BEAM.

R. SHEATUNG.

I. ROOF SHEATHING COVERED BY COMPOSITE ROOFING SHALL. BE A MINIMUM OF 15:32 INCH THICK (NOMINAL.) O.S.B. MANUFAC TURED WITH EXTERIOR GLUH.

II. ROOF SHEATHING COVERED BY THE SHALL BE A MENIMUM
OF 5/8 INCH THICK (NOMINAL.) MANUFACTURED WITH EXTERIOR

GLUF.

III. THE LONG SIDE OF THE SHEATHING SHALL BE INSTALLED PERPENDICULAR TO THE ROOF TRUSS SYSTEM.

IV. FASTENING SHALL BE SARING SHANK NAH'S AT 4 INCHES ON CENTER AT BOUNDARY AND BIXES AND 6 INCHES ON CENTER IN THE FIELD WITH A SETBACK OP 5 -0° FROM ALL BIXES.

IV. METAL THE CONSULTATION OF DECORPORATION SHALL BE USED.

V. METAL "H" CLIPS OR SOLID WOOD BLOCKING SHALL BE USED AT ALL UNSUPPORTED BUTT JOINTS BETWEEN TRUSSES OR RAFTERS.
25. PRECAST CONCRETE LIMITIES.

25. PRECAST CONCRETE LINTELS

A. PRECAST AND PRESTRESSED CONCRETE LINTELS SHALL BE MANUFACTURES SPECIFICATIONS AND INSTRUCTIONS.

B. THE SIZE OF THE LINTELS SHALL. BE BASED ON THE SPAN AND LOAD. REFER TO THE ATTACHED SCHEDULE UNITESS OTHERWISE SHOWN IN THE STRUCTURAL DESIGN FOR THE SPECIFIED LINTEL.

C. LINTEL SCHEDULE UNIO. ON PLANS:

I. SPAN UP TO 3"- 8F8-OB

II. SPAN UP TO 3"- 14"- 8F8-OB

III. SPAN 6" TO > 14"- 8F8-OB

III. SPAN 6" TO SHE SERVER STRENGTH TO BE USED FOR LINTELS IS 3,000 PSI.

FOR LINTELS IS 3,000 PSI.

THE REINFORCING STEEL SHALL BE ASTM GRADE 60

26. FASTENERS / METAL CONNECTORS.

A. ALL FASTENERS AND METAL CONNECTORS SHALL BE MANUFACTURED BY SIMPSON STRONG TIE AND INSTALLED PER THE MANUFACTURES SPECIFICATIONS AND INSTRUCTIONS.

B. THESE FASTENERS DO NOT INCLUDE TYPICAL NAILS AND SCREWS WHICH MAY BE MANUFACTURED BY OTHERS.

C. FOLLOW ALL MANUFACTURES SPECIFICATIONS AND INSTRUCTIONS FOR

MAY BE MANUFACTURED BY OTHERS.
C. FOLLOW ALL MANUFACTURES SPECIFICATIONS AND INSTRUCTIONS FOR ALL FASTEBERS, METAL CONNECTIONS, SCREWS, NAB S, ETC. THAT ARE IN CONFACT WITH PRESSURE TREATED LUMBER.

27. DIMENSIONAL LUMBER:

A. ALL LOAD BEARING WALLS SHALL BE SOUTHERN YELLOW PINE #2 OR BETTER GRADED AND STAMPED BY THE CERTIFYING AGENCY. IN ADDITION, ALL WIXID SHALL BE PRESSURE TREATED FOR EXTERIOR USE WHERE EXPOSED TO MOISTURE, PLACED WITHIN 12 INCHES OF SOIL OR IN CONTACT WITH CONCRETE OR MASONRY.

28. STRUCTURAL SHEATHING:

A. ALL SHEATHING USED FOR EXTERIOR APPLICATIONS SHALL BE EXTERIOR GRADE AND ADA STAMPED AND VERIFYING ITS RATING.

19. MASONRY:

A. CONCRETE MASONRY UNITS SHALL CONFORM WITH AMERICAN MASONRY INSTITUTE STANDARD 530

B. CONCRETE MASONRY UNITS SHALL HAVE A MINIMUM COMPRESSIVE

STRENGTH OF 1900 PSI

C. MORTAR SHALL BE OF TYPE M OR S GRAY MORTAR

A. ALL GROUT SHALL BE A FINE TYPE HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI UNLESS SPECIFICALLY SHOWN OTHERWISE BY A MANUFACTURER PURSUANT TO GROUP USE WITH ITS PRODUCTS.

31. REINFORCING STEEL:

A. ALL REINFORCING STEEL SHALL BE ASTM GRADE 40 EXCEPT GRADE 60 SHALL BE USED FOR GRADE BEAMS, ALL LINTEL TYPES (LE. PRECAST AND FIELD PREFORMED) COLUMNS UNLESS OTHERWISE SHOWN IN THE STRUCTURAL PLANS.

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- 32. STRUCTURAL STEEL AND CONNECTION ACCESSORY MATERIAL:
 A. I-BEAMS, FORMED STRUCTURAL STEEL, FLAT BAR OR PLATE
 SHALL BE ASIM GRADE A36 UNLESS STATED OTHERWISE,
 B. ALL STRUCTURAL STEEL SHALL HAVE A MINIMUM OF TWO
- COATS OF PRIMER AND TWO COATS OF EPOXY AS A CORROSION PREVENTIVE. THE BUILDING CONTRACTOR MAY VARY FROM THIS SPECIFICATION WITH THE APPROVAL OF THE VARY FROM THIS STEELED ATTOM WITH THE APPROVAL OF THE STRUCTURAL ENGINEER IF IT CAN BE DEMONSTRATED ANOTHER MEANS OF CORROSION CONTROL IS EQUALLY EFFECTIVE.

 C. ALL WELDING OF STRUCTURAL STEEL SHALL BE MADE WITH F60/70 TYPE ELECTRODES. THE DEPTH AND LENGTH FOR THE WELD
- SHALL BE SPECIFIED IN THE STRUCTURAL DESIGN FOR THE SPECIFIC CONNECTION.
- 33. VENTILATION:
- S. VENTILATION:

 THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR DETERMINING VENTILATION REQUIREMENTS OF CRAWL SPACES, FLOORS AND ATTICS NOR THE MEANS AND METHODS FOR IMPLEMENTING THESE REQUIREMENTS.
- 34. WATERPROOFING:
- ANY RENDERING OF NOTES OF WATERPROOFING MEASURIS FOR BASEMENTS OR HALF BASEMENTS SHOWN IN THESE PLANS WHERE A SPECIFIC CONSTRUCTION DETAIL IS NOT SHOWN IN THE STRUCTURAL A SPECIFIC CONSTRUCTION DEFAIL IS NOT SHOWN IN THE STRUCTURAL DESIGN IS AN ARCHITECTURAL ILLUSTRATION ONLY AND IS NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER,

 B. CRICKETS ARE ASSOCIATED WITH THE ARCHITECTURAL FINISHES AND ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER,
- 35. FIRE RESISTANT DESIGN:
- A. FIRE RESISTANT DESIGN OF STRUCTURAL ELEMENTS SHALL BE INCIDENTAL TO THEIR STRUCTURAL DESIGN AND SHALL BE BASED ON UNDERWRITERS LABORATORY OR GYPSUM ASSOCIATION DESIGN FOR FIRE RATED FLOOR, WALL AND ROOF ASSEMBLIES. 36. FLOOD RESISTANT DESIGN:
- A. FLOOD RESISTANT DESIGN OF FLOOD RESISTANT DESIGN OF STRUCTURAL ELEMENTS SHALL BE INCIDENTAL TO THEIR STRUCTURAL DEIGN AND SHALL BE BASED ON THE REQUIREMENTS STATED IN TITLE 44 CFR SECTIONS 59 AND 60, AND ON THOSE OF THE INDIVIDUAL COMMUNITY
- SECTIONS 59 AND 60, AND ON THOSE OF THE INDIVIDUAL COMMUNITY
 RATING AGENCIES FOR THE GOVERNMENTAL JURISDICTION WHERE THE
 CONSTRUCTION IS TO BE DONE.
 B. HOWEVER, THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR
 IDENTIFYING AND SHOWING ON THE PLANS THE FLOOD ZONE CATEGORY,
 BASE FLOOD ELEVATION, AND THE FLOOR AND STORY HEIGHTS OF THE
 BUILDING IN RELATION TO THE BASE FLOOD ELEVATION. THIS INFORMATION
 IS CONSIDERED ARCHITECTURAL AND SITE RELATED AND SHALL BE PROVIDED TO THE STRUCTURAL ENGINEER BY THE CONTRACTING CLIENT

- PROVIDED TO THE STRUCTURAL ENGINEER BY THE CONTRACTING CLIEN OR HIS AGENT.

 37. SPECIAL CONSTRUCTION:
 1. ALUMINUM STRUCTURAL COLUMNS,
 1. ALUMINUM STRUCTURES SHOWN IN THESE PLANS SUCH AS PORCH AND POOL ENCLOSURES OR GUARDRAILS AND HANDRAILS ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL
- ENGINGER.

 B. WHERE THE ALUMINUM STRUCTURE ATTACHES TO THE MAIN STRUCTURE OR IS INCORPORATED IN THE MAIN STRUCTURE, SHOP DRAWINGS FOR THESE STRUCTURES SHALL BE PROVIDED TO THE STRUCTURAL ENGINEER TO DETERMINE THEIR EFFECT ON THE MAIN STRUCTURE.
- A. ANY SWIMMING POOL OR HOT TUBS SHOWN IN THESE PLANS ARE FOR
- A. ANY SWIMMING POOL OR HOT TUBS SHOWN IN THESE PLANS ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL DESIGN. IIL FENCES AND RETAINING WALLS:

 A. ANY RENDERING OF FENCES. RETAINING WALLS OR EXTERIOR PLANTERS WHERE A SPECIFIC STRUCTURAL DETAIL IS NOT SHOWN FOR THEIR CONSTRUCTION ARE FOR ARCHITECTURAL ILLUSTRATION ONLY AND ARE NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER. IV. DRIVEWAYS AND WALKWAYS:
- IV. DRIVEWAYS AND WALGERATE:

 A. ANY DRIVEWAYS OR WALKWAYS SHOWN IN THESE PLANS ARE FOR ARCHITECTURAL ILLUSTRATION PURPOSES ONLY AND ARE NOT PART OF THE STRUCTURAL DESIGN OR THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.

Floor and Roof Live Londs				
Attics:	20 psf w/ storage, 10 psf w/o storage			
Habitable Attics, Louisvani.	30 psf			
All Other Rooms:	40 psf			
Garage:	40 psf			
Roofs:	20 psf			
	Wind Design Date			

Wind 1	Design Data			
Ultimate Wind Speed:	145 mph			
Nominal Wind Speed:	112 mph			
Risk Category:	11			
Wind Exposure:	B			
Enclosure Classification:	Enclosed			
Internal Pressure Coefficient:	0,18 +/-			
Components and Cladding Design Pressu	res:			
Roofing Zone 1:	+16.0 psf max., -20.7 psf min.			
Roofing Zone 2:	+16.0 psf max., -36.0 psf min.			
Roofing Zone 3;	-53.2 psf min.			
Roofing at 2	Zone 2 Overhangs: -42.1 psf min.			
Roofing at 2	one 3 Overhangs: -70.9 psf min.			
Stucco, Cladding, Doors & Windows:				
	+22.6 psf max., -24.5 psf min.			
Zone 5:	+22.6 psf max., -30.2 psf min.			
End Zone Width:	4,00 ft.			
The Nominal Wind Speed was used to c Cladding Design Pressures.	letermine the above Component and			
All exterior glazed openings shall be pro Section 1609.1.2 of the 2017 FBC.	otected from wind-borne debris as per			
The site of this building is not subject to	special topographic wind effects as per			

Geotechnical Infor	mation
Design Soll Load-Bearing Capacity:	2,000 psf
Flood Design D	ata
Flood Zone:	X

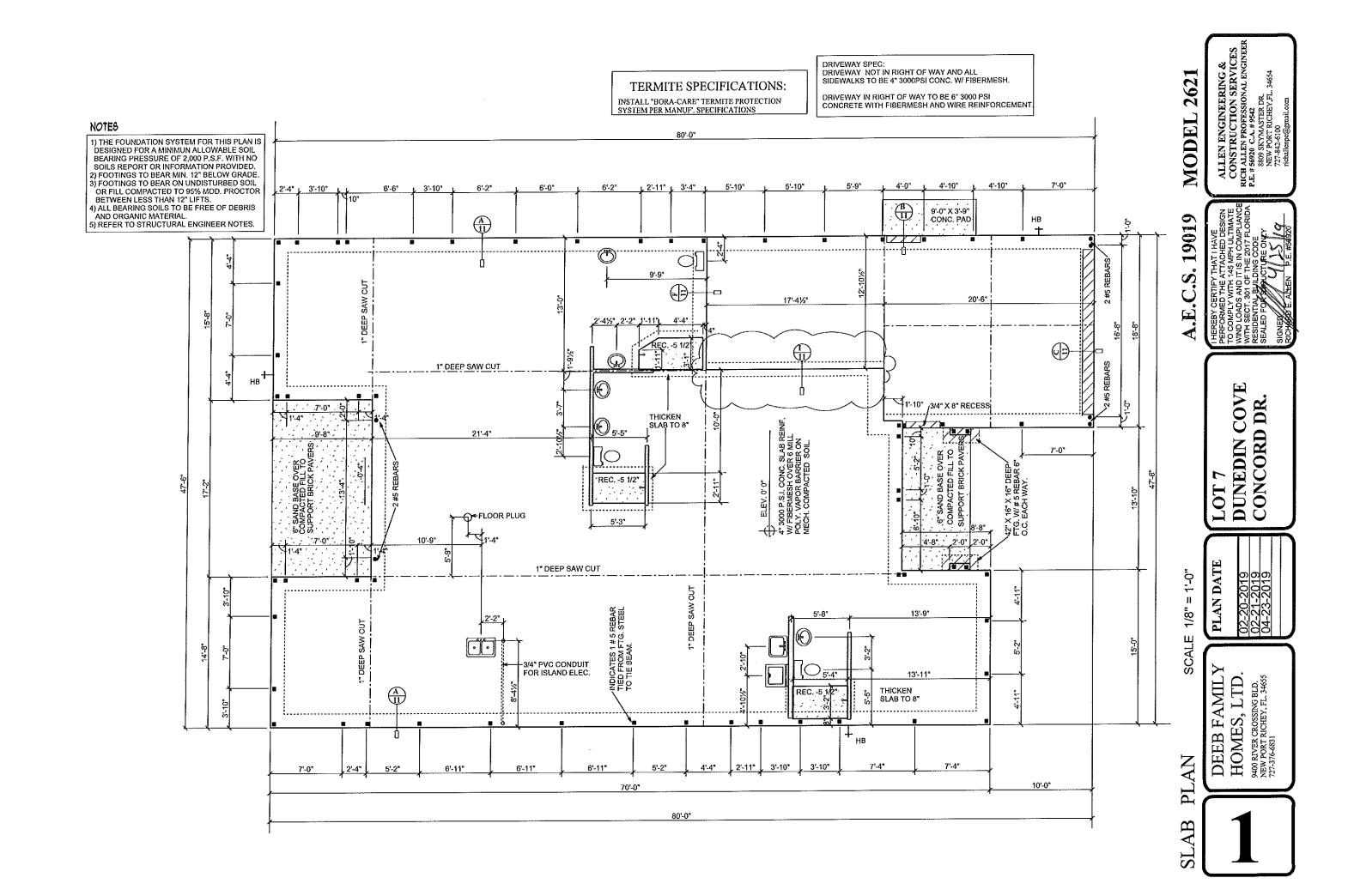
Section 1609.1.1.1 of the 2017 FBC.

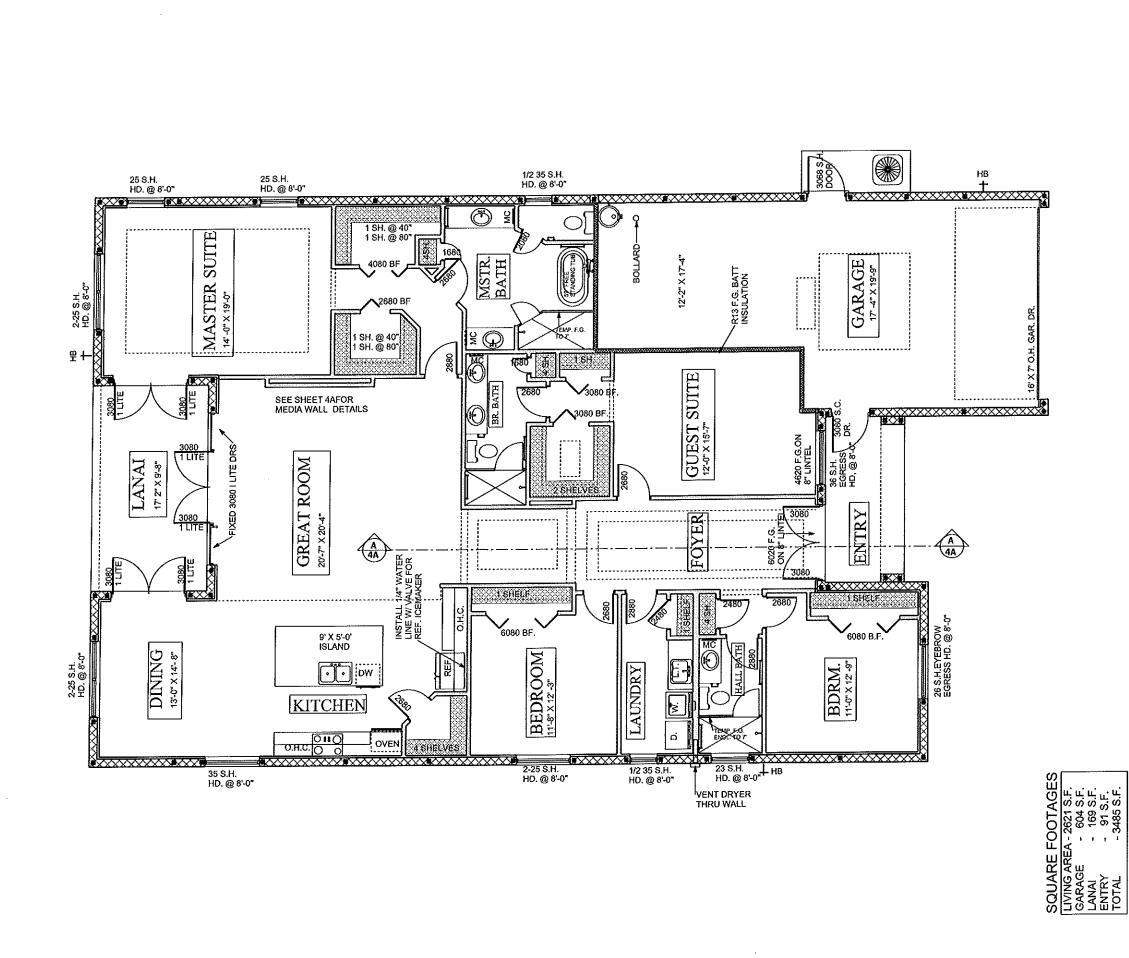
* 0.6 ALLOWABLE STRESS DESIGN USED *

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PLAN DATE WIND LOAD DESIGN DATA

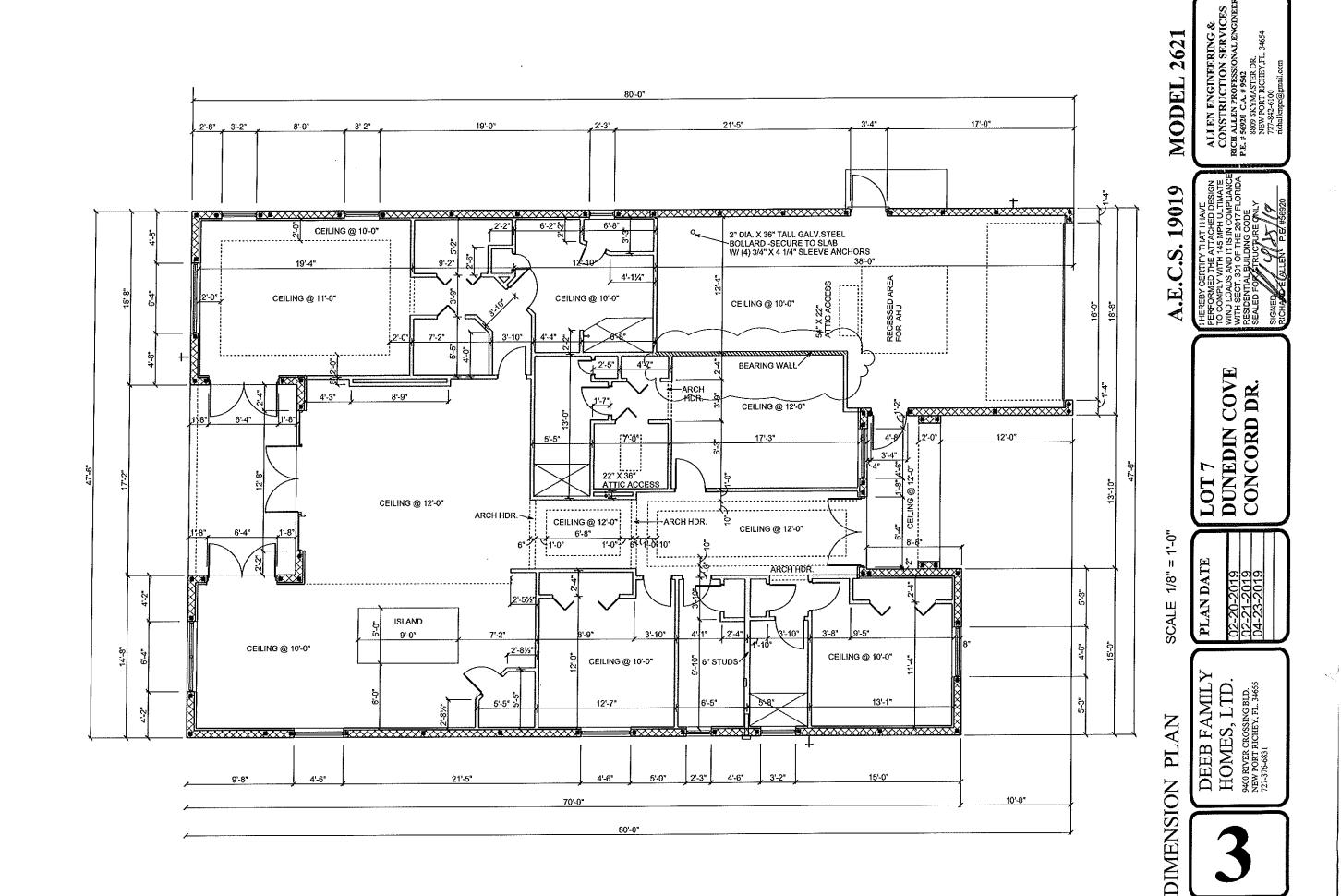
DEEB FAMILY HOMES, LTD. 8400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 14655 72776-6831

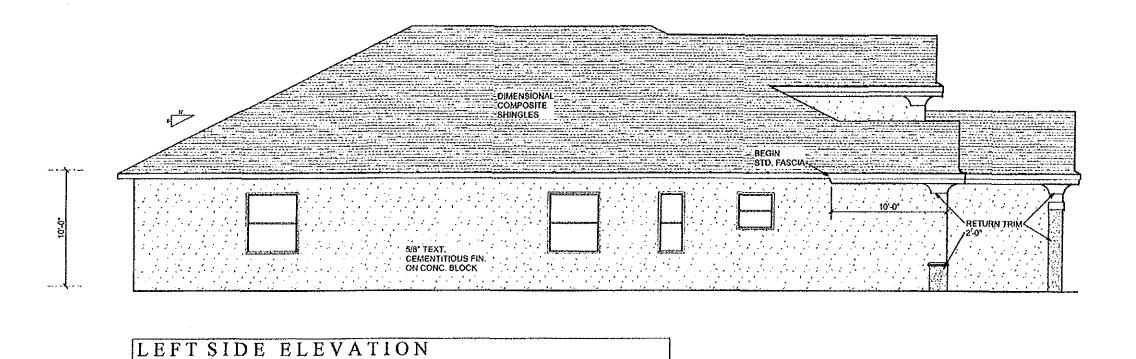


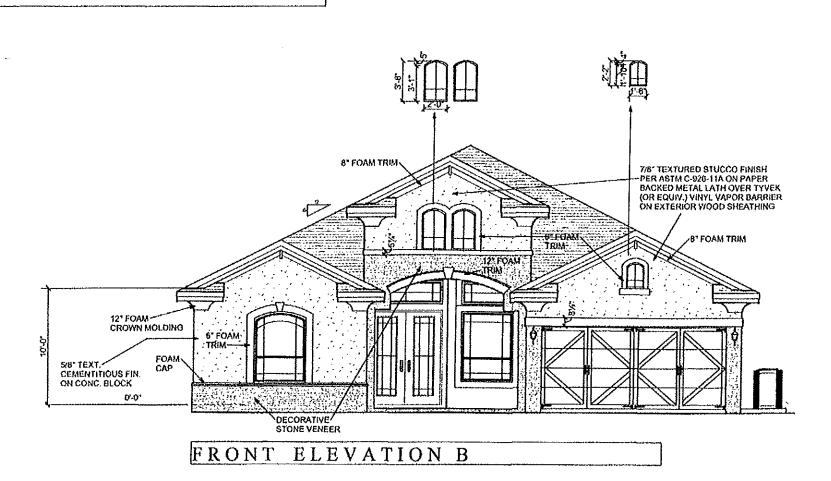


MODEL A.E.C.S. 19019 DUNEDIN COVE CONCORD DR. LOT 7 1. 0. PLAN DATE SCALE 1/8" DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831 PLAN NOTES FLOOR

2621

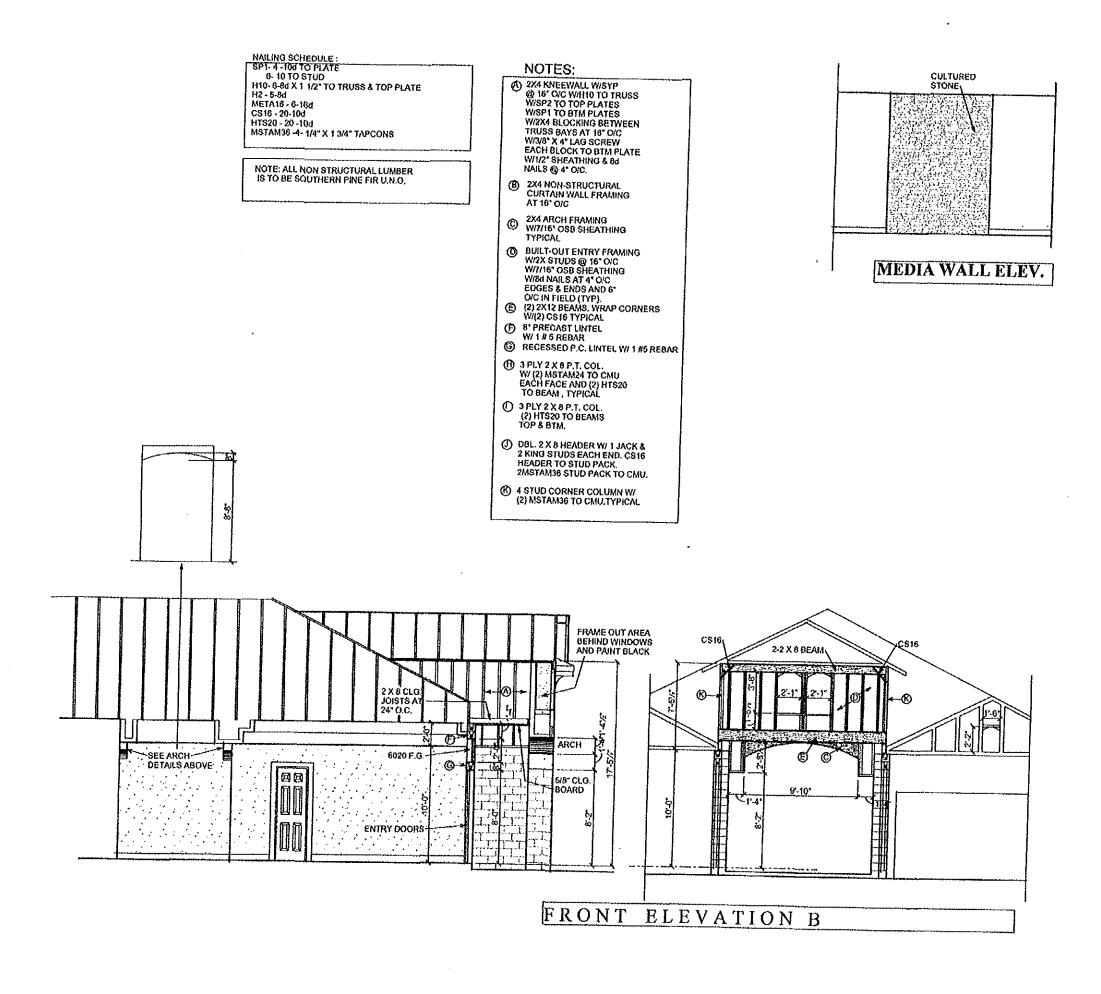






MODEL 2621 A.E.C.S. 19019 LOT 7 DUNEDIN COVE CONCORD DR. 1/8" PLAN DATE ELEVATIONS DEEB FAMILY HOMES, LTD.

WOW RIVER CROSSING BLD.
NEW PORT RICHEY, FL. 34655
727-376-6531 EXTERIOR



1/8" SCALE & EXTERIOR DETAILS INTERIOR

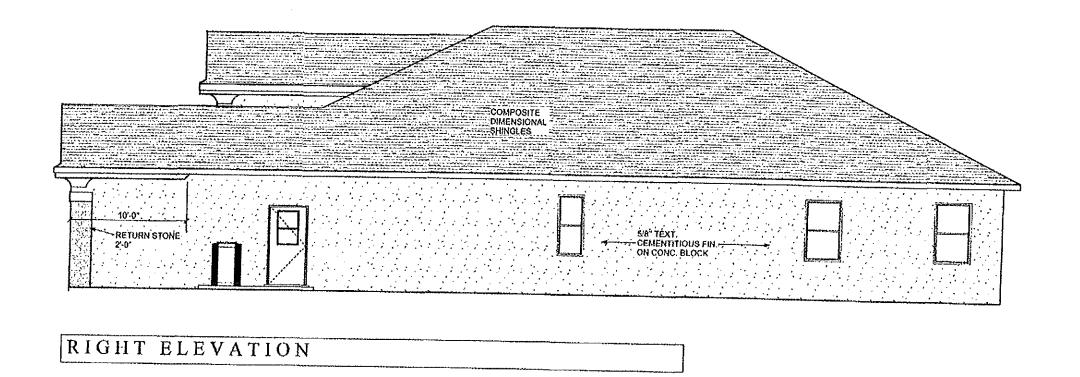
19019 A.E.

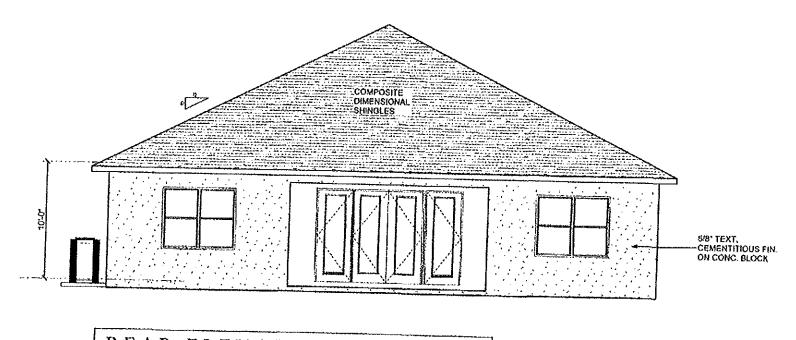
MODEL 2621

LOT 7 DUNEDIN COVE CONCORD DR.

PLAN DATE

DEEB FAMILY HOMES, LTD. 400 RIVER CROSSING BLD. NEW FORT RICHEY, PL. 34655





MODEL 2621

A.E.C.S. 19019

SCALE 1/8" = 1'-0"

EXTERIOR ELEVATIONS

PLAN DATE

DEEB FAMILY HOMES, LTD. 9400 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 27-376-6831

LOT 7 DUNEDIN COVE CONCORD DR.

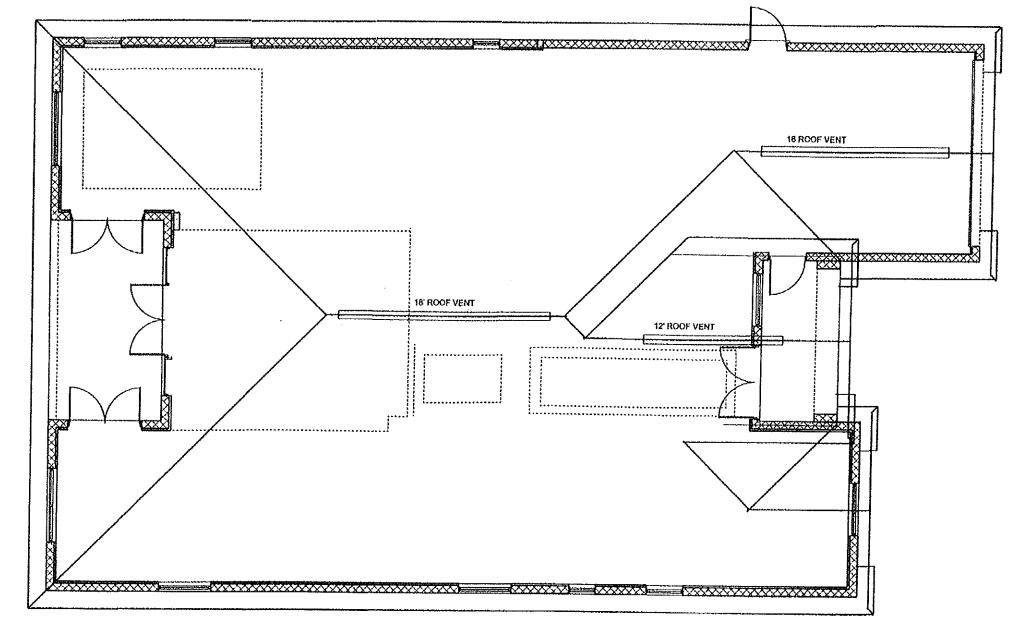
REAR ELEVATION

TOTAL NET FREE VENTILATING AREA SHALL NOT BE LESS THAN 1 TO 300 PROVIDED THAT AT LEAST 50 % AND NOT MORE THAN 80 % IS PROVIDED BY VENTILATORS LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED PER SECT. R836.2

TOTAL AREA TO BE VENTILATED = 3485 S.F. 3485/300 = 11.6 S.F. OR 1870 SQUARE INCHES.

ROOF VENTS ARE RATED AT 36 SQUARE INCHES OF OPENING PER LINEAL FT, 1670 S.I./38 S.1. #46 LINEAL FEET REQUIRED.

INSTALLATION FOR THIS ROOF IS 46 OF ROOF VENTING

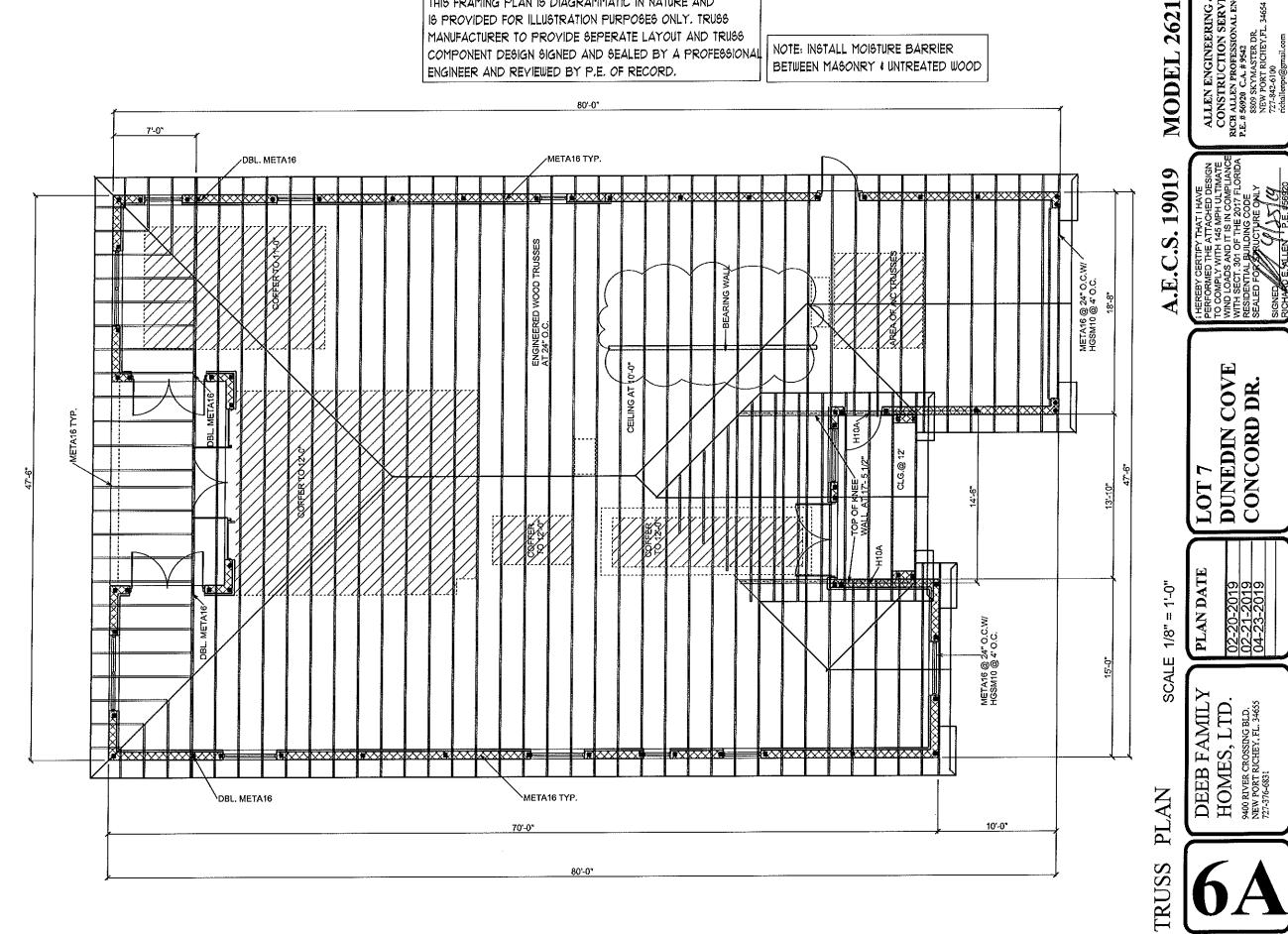


MODEL 2621 19019 LOT 7 DUNEDIN COVE CONCORD DR. PLAN DATE 1/8" = 1'-0" DEEB FAMILY HOMES, LTD. WOW RIVER CROSSING BLD. NEW PORTRICHEY, PL. 34655 275-376-6831 PLAN ROOF

IMPORTANT NOTE:

THIS FRAMING PLAN IS DIAGRAMMATIC IN NATURE AND IS PROVIDED FOR ILLUSTRATION PURPOSES ONLY. TRUSS MANUFACTURER TO PROVIDE SEPERATE LAYOUT AND TRUSS COMPONENT DESIGN SIGNED AND SEALED BY A PROFESSIONAL ENGINEER AND REVIEWED BY P.E. OF RECORD.

NOTE: INSTALL MOISTURE BARRIER BETWEEN MASONRY & UNTREATED WOOD



UNLESS OTHERWISE NOTED

1. ELECTRICAL OUTLET HEIGHTS MEASURED FROM FINISHED FLOOR
TIO CENTERLINE OF THE BOX TO BE 18" A.F.F. (GENERAL)
KITCHEN 42"
BATHROOM 42"
LAUNDRY 36" WASHER! 24" DRYER! WALL OUTLETS 45"
EXTERIOR WATERPROOF @ 12"
GARAGE GFI @ 45"
RANGE 220V @ 4"

2. ALL TRIM PLATES AND DEVICES TO GANGED WHERE POSSIBLE
3. ELECTRICAL SWITCHES TO BE AT 42" CENTERLINE A.F.F. 4. ELECTRICAL PLAN IS INTENDED FOR BID PURPOSES ONLY, ALL WORK SHALL, BE DONE IN STRICT ACCORDANCE WITH THE NATIONAL ELECTRICAL CODE, LATEST EDITION BY A LICENSED ELECTRICAL CONTRACTOR WHO SHALL BE RESPONSIBILE FOR THE INSTALLATION & SIZING OF ALL ELECTRICAL, WIRING & ACCESSORIES.

5. SMOKE DETECTORS SHALL BE IN ACCORDANCE WITH THE FLORIDA BUILDING CODE, SECTION R314

6. PROVIDE AFCI (ARC FAULT INTERRUPTERS) IN ALL BEDROOMS PER NEC, SECTION 210-12

7. ALL RECEPTICALS TO BE TAMPER PROOF PER SECT. 406.11 2621 MODEL 19019 LOT 7
DUNEDIN COVE
CONCORD DR. **ELECTRICAL LEGEND** SMOKE DETECTOR / CARBON MONOXIDE DETECTOR SPAGLE POLE SYMICH OUTLETS FLOOD LIGHT DOUBLE POLE SWITCH THREE-WAY SWITCH XXXXXXXXXXXXXXXX FLUDRESCENT LIGHTENS FOUR-YAY SWITCH CHITHOUNDARY THE THE COO CENTROLM SCOUNCE (WALL SKOUNTED) FOXTURE HO VOLT OUPLEX OUTLET HIS VOLT EPHT SWITCHED OUTLET DOOR BELL CHAIRS GROUND FAULT INTERRUPT 0003 800 WP WATER PROOF WE GROUND FAULT ō DISPOSAL SPECIAL SERVICES CUTTER DISCONNECT ENVIRON \$P PREWIRE SPEAKER TELEPHONE CASLE OUTLET <u>=</u>8 AMCTIONEDX RECESSED LIGHTEIO 0 THERMOSTAT WATER PROOF RECESSED LIGHTERS LOWYOUTAGELIGHTING (C) METERCOM SYSTEM GARAGE 00003 PUSH BUT (OH DEEB FAMILY HOMES, LTD. 9600 RIVER CROSSING BLD. NEW PORT BICIEN, FL. 34655 127-376-6531 ELECTRICAL PLAN

\$04 CHANERSWICH CELLING FIXTURE

220 VOLT OUTLET

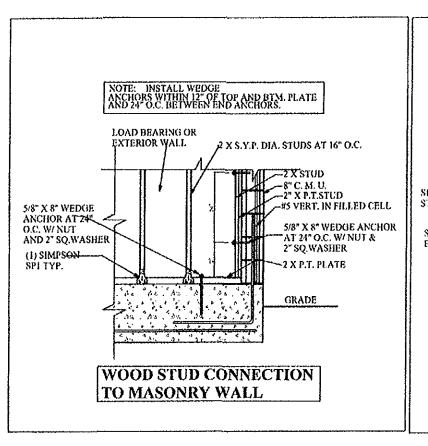
EV. CABLE OUTLET

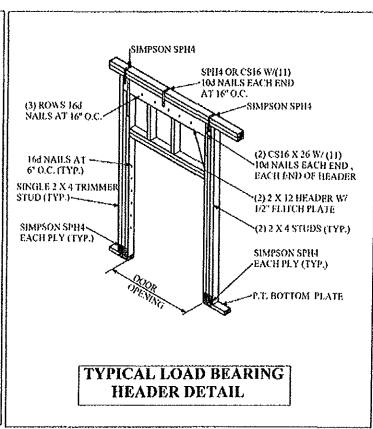
BATRIEAN WASAIT

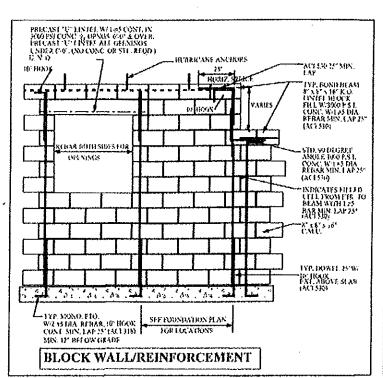
BATHFAN

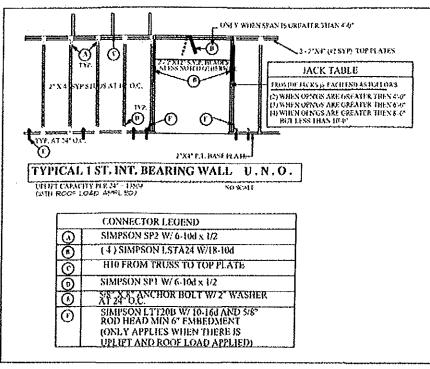
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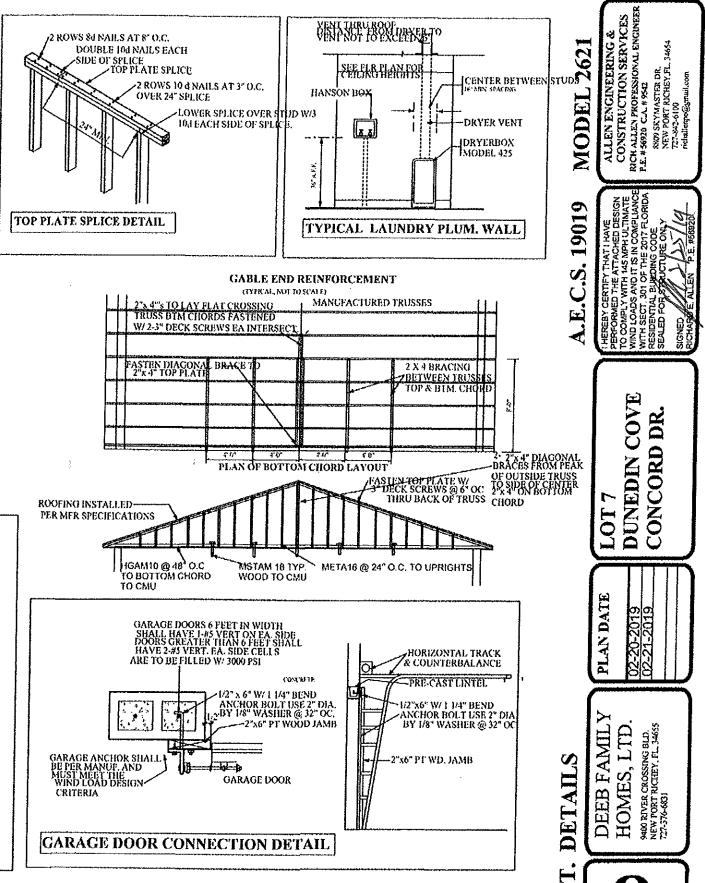
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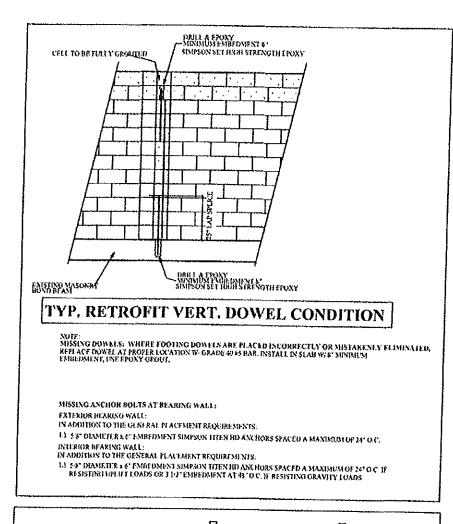


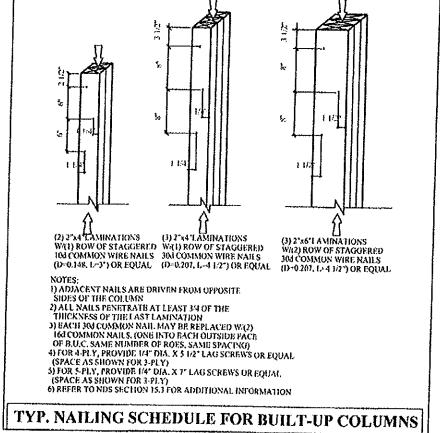


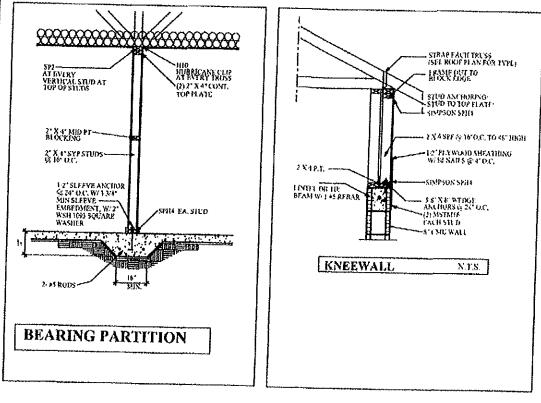


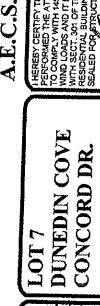












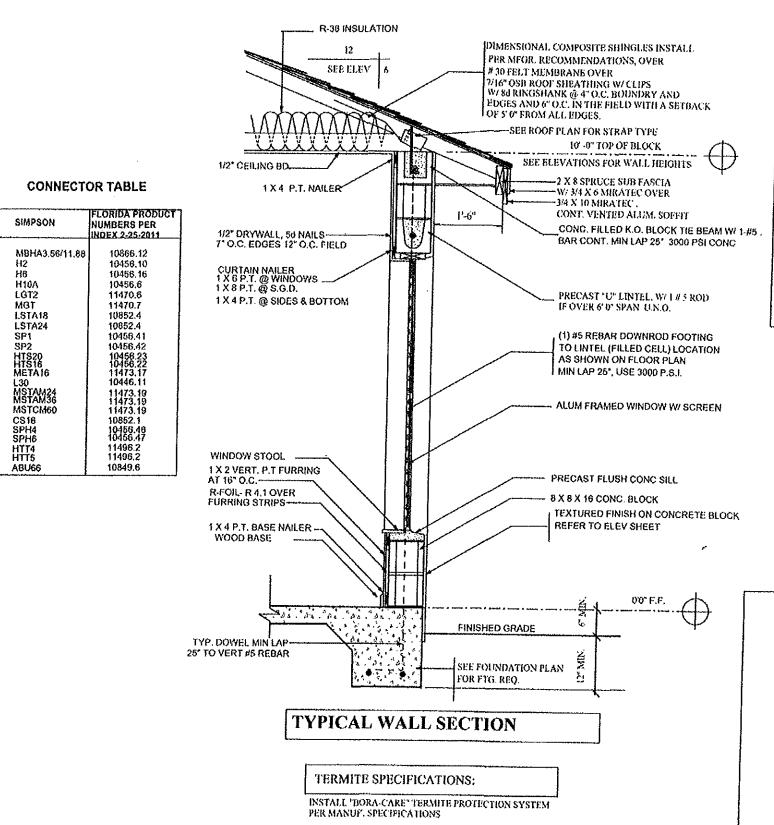
PLAN DATE

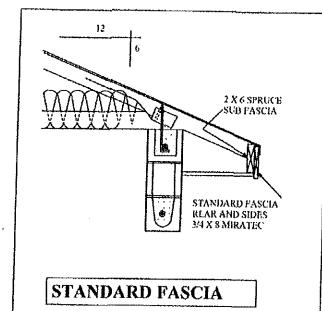
DEEB FAMILY HOMES, LTD. 9401 RIVER CROSSING BLD. NEW PORT RICHEY, FL. 34655 727-376-6831

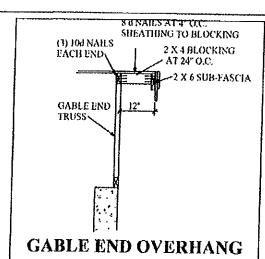
CONST. DETAILS

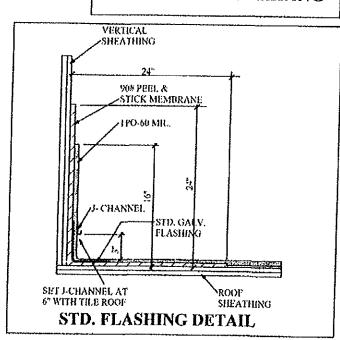
MODEL

19019







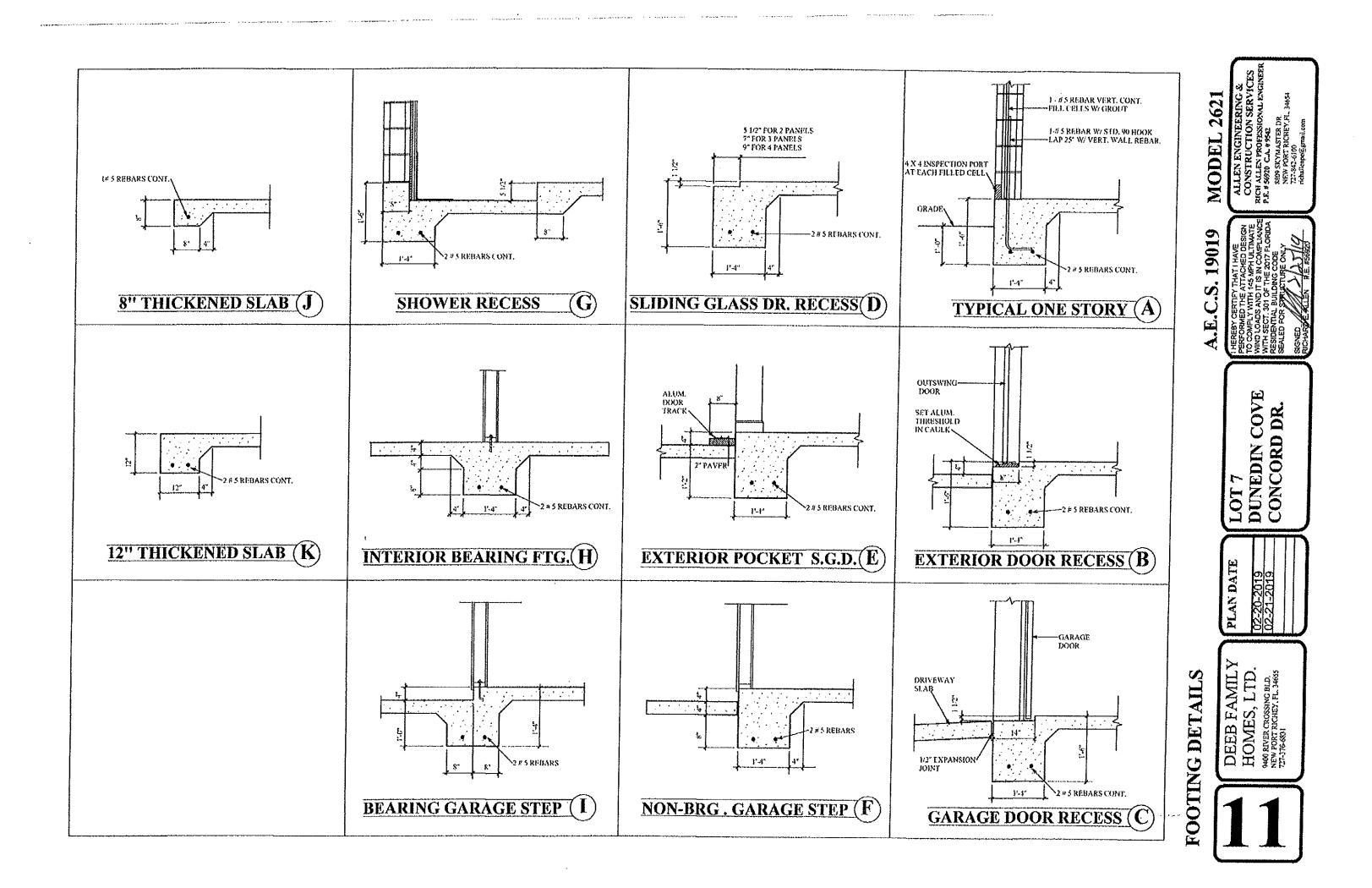


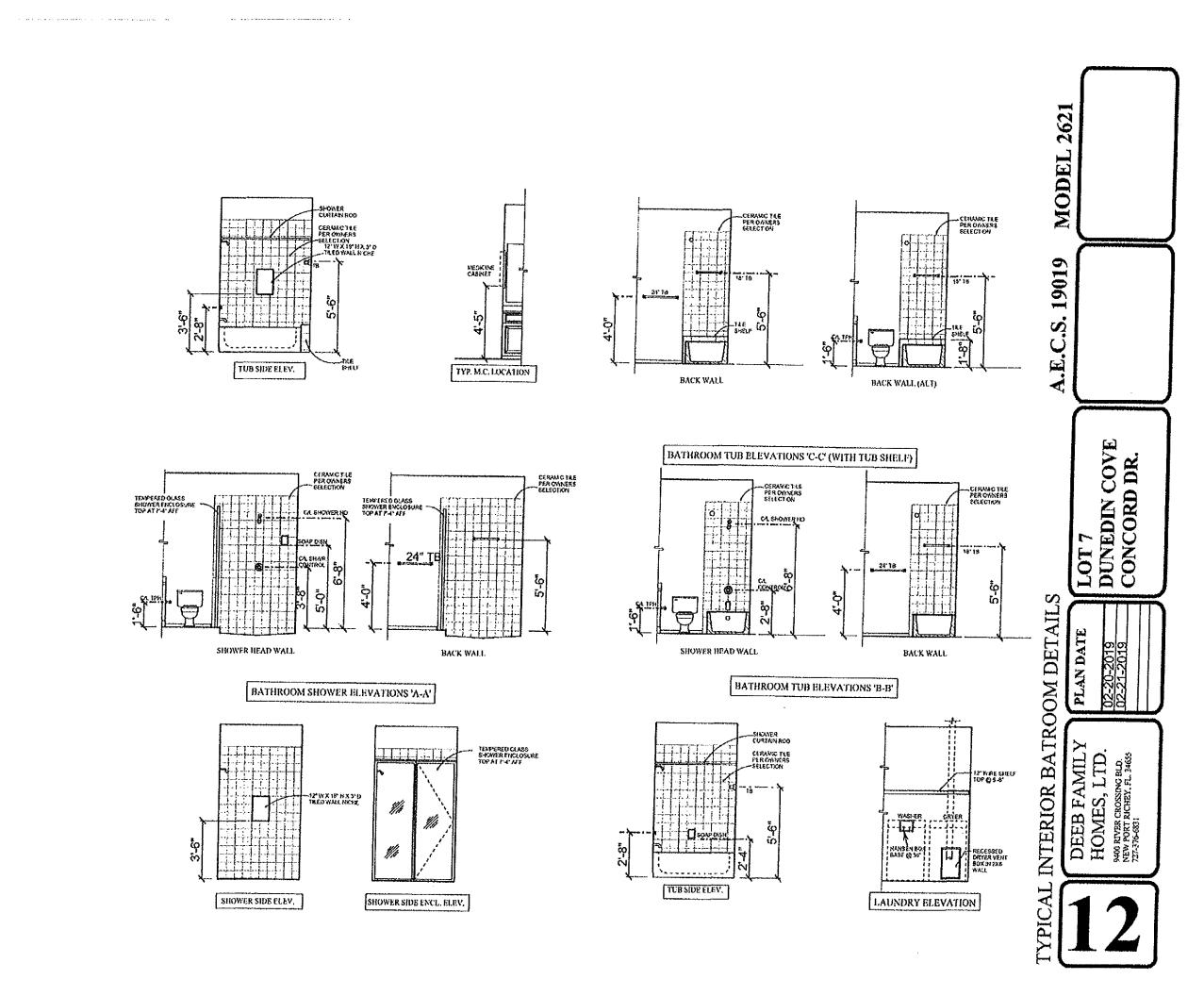
DEEB FAMILY HOMES, LTD.
9400 AUVER CROSSING BLD.
NEW PORT BICHEY, FL. 24655
227-376-6531 DETAILS

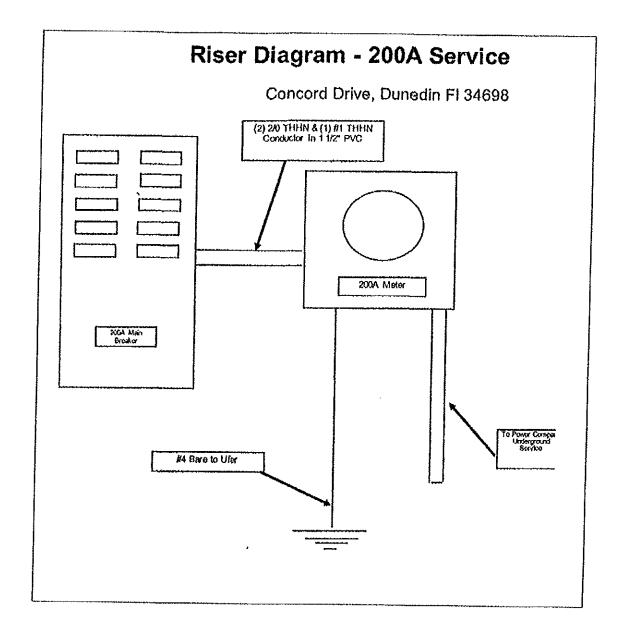
MODEL

DUNEDIN COVE CONCORD DR. LOT

PLAN DATE







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		Calcula					
Project Inform	ation:					Drive	
	-	Duneo	lin	, F	34698		
Description		Qty,			Qty.	Watts	
Sq. Ft. x 3 Watts		272	25	X	3	8175	
Small Appliance Branch			4	Х	1500	6000	
Laundry			1	X	1500	1500	
Disposal			1	Х	1080	1080	
Dishwasher			1	х	1300	1300	
Range		7.	1	x	8000	8000	
Oven				x	9600	0	
Cook Top				x	9000	0	
Jen Air				x	7680	0	
Water Heater			7	x	4500	0	
Dryer			1 /	x	5000	5000	
Microwave			1)	ĸ	1200	1200	
Jacuzzi			7	7	2400	0	
Pool			>	त	1200	0	
			×	7	7200	0	
Pool Heater			Х		14400	0	
Bath Fans		4	l x	T	60	240	
			Х		60	0	
			S	ub	Total =	32,495.00	
						(10,000.00)	
Sub Total = x .40% Sub Total =						22,495.00	
						x .40%	
						8,998	
	·	-		-		10,000.00	
C Name plate or 4 x Sq Ft	····			-			
C#1		2725	X		4	10900	
C#2			X			0	
C#3		· · · · · · · · · · · · · · · · · · ·	x			0	
H (KW + 1000 + Fan)				••			
H#1	8k	W				8900	
H#2						. 0	
H#3						0	
V Charging station					12000	0	
	·	Total Watts =				38,798.00	
Divided by						240	
		To	ta	ΙĀ	mps =	162	

Main Breaker Size

200

MODEL 2621 A.E.C.S. 19019 LOT 7 DUNEDIN COVE CONCORD DR. PLAN DATE ELECTRICAL RISER & LOAD CALCULATIONS