

MECAWind Version 2.1.1.4 per ASCE 7-10

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Date : 9/25/2012 Project No. : DHD 2084 PLAN
 Company Name : Paul H Hagler, Consulting Engr. Designed By : Paul H Hagler, FPE 20158
 Address : 1172 Fernwood Description : New Residence
 City : Holiday Customer Name : Deeb Family Home
 State : FL 34680 Proj Location : Safety Harbor, FL
 File Location : D:\CADD\Wind\2012\1084\2084.DHD\2084.Plan.mcd

Directional Procedure Simplified Diaphragm Building (Ch 27 Part 2)

All pressures shown are based upon ASD Design, with a Load Factor of 1.6

Basic Wind speed (v)	115.00 mph	Exposure category	B
Risk Category	II	Flexible structure	No
Natural frequency	N/A	Kd Directional Factor	0.85
Importance Factor	1.00		
Drapping ratio (beta)	0.41		
Alpha	7.00	Zg	1200.00 ft
Kt	0.14	Rt	0.84
Kz	0.25	Km	0.45
Cd	0.39	I	320.00 ft
Epsilon	0.32	Zmin	30.00 ft
Slope of Roof	4 : 12	Slope of Roof (theta)	18.42 Deg
Rt: Mean Roof Rt	15.58 ft	Type of Roof	Hipped
RHt: Ridge Ht	21.16 ft	Ekt: Eave Height	10.00 ft
OH: Roof Overhang at Eave	1.50 ft	Roof Area	5155.00 ft ²
ldg Length Along Ridge	70.00 ft	ldg Width Across Ridge	64.00 ft

Gust Factor Summary
 Not a Flexible Structure use the Lesser of Gust1 or Gust2 = 0.95

Table 26.11-1 Internal Pressure Coefficients for Buildings, GCp1
 GCp1 : Internal Pressure Coefficient = +/- 0.10

Wind Pressure on Components and Cladding (Ch 30 part 1)
 All pressures shown are based upon ASD Design, with a Load Factor of 1.6

Width of Pressure Coefficient Zone "a" = 6.232 ft

Description	Width ft	Span ft	Area, Zone ft ²	Max GCp	Min GCp	Max P psf	Min P psf
10 sf Interior	2.00	5.00	10.00	0.00	-1.10	22.69	-24.62
10 sf Corner	2.00	5.00	10.00	0.00	-1.40	22.69	-30.39
20 sf Interior	4.00	5.00	20.00	0.95	-1.05	21.67	-23.59
20 sf Corner	4.00	5.00	20.00	0.95	-1.29	21.67	-28.34
50 sf Interior	8.00	5.00	40.00	0.89	-0.99	20.65	-22.57
50 sf Corner	8.00	5.00	40.00	0.89	-1.19	20.65	-26.30
100 sf Interior	14.30	7.00	100.14	0.82	-0.92	19.30	-21.22
100 sf Corner	14.30	7.00	100.14	0.82	-1.05	19.30	-23.59
200 sf Interior	28.60	7.00	200.28	0.77	-0.87	18.27	-20.20
200 sf Corner	28.60	7.00	200.28	0.77	-0.94	18.27	-21.55
Wall Interior	20.00	10.00	200.00	0.77	-0.87	18.27	-20.20
Wall Corner	6.23	10.00	62.31	0.86	-1.12	19.99	-24.99
Roof Interior	10.00	10.00	100.00	0.30	-0.80	16.00	-18.85
Roof eave	30.00	6.23	187.02	0.30	-1.20	16.00	-26.54
Roof corner	6.23	3.00	18.70	0.45	-1.56	16.00	-33.54
Roof Eave overhang	30.00	1.50	45.00	0.37	-2.20	16.00	-42.31
Roof corner overhang	6.23	1.50	9.34	0.50	-3.70	16.00	-71.16

Xhcc: Comp. & Clad. Table 6-1 Case 1 = 0.76
 Ghcc: 0.0255 * V² * Xhcc * Kzt * Kd = 19.23 psf

ENGINEER'S NOTES:

ENGINEER RESERVES THE RIGHT TO MAKE MODIFICATIONS AS CONSTRUCTION PROGRESSES. THIS IS A REMODEL TO AN EXISTING HOME, AND AS SUCH, EXISTING CONDITIONS MUST BE ASSESSED BY CONTRACTOR. QUESTIONABLE AREAS MUST BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD. ENGINEER HAS DONE NO FIELD INVESTIGATION PRIOR TO CONSTRUCTION. EXISTING CONDITIONS INDICATED IN THIS PLAN SET ARE ASSUMED. CONTRACTOR TO INFORM ENGINEER OF ANY DIFFERENCE IN DIMENSION OR BEARING AS DEMOLITION COMMENCES. ENGINEER MAY REQUIRE ADDITIONAL SUPPORT OR MAY MAKE CHANGES TO ATTACHMENT CRITERIA AS WORK PROGRESSES. CONTRACTOR IS NOT TO ASSUME ANYTHING, BUT IS TO CONSULT WITH ENGINEER FOR CLARIFICATION OF ANY PLAN NOTES OR FIELD CONDITION FOUND TO DIFFER FROM THOSE PRESENTED IN THE PLANS.

ALL PRE-ENGINEERED WOOD PRODUCTS ARE THE RESPONSIBILITY OF THE TRUSS MANUFACTURER. THE TRUSS ENGINEER IS A DELEGATED ENGINEER FOR THIS PROJECT, AND AS SUCH, IS RESPONSIBLE FOR THE VALIDITY OF THE COMPONENTS PROVIDED. FRAMING LAYOUTS SHOWN MAY BE CHANGED BY THE TRUSS MANUFACTURER. THE DELEGATED ENGINEER IS RESPONSIBLE FOR PROVIDING A FINAL SEALED SET OF ALL CALCULATIONS AND LAYOUTS FOR THIS PROJECT TO THE ENGINEER OF RECORD FOR REVIEW PRIOR TO MANUFACTURE OF SAID COMPONENTS. ENGINEER OF RECORD HAS NOT REVIEWED THE PRE-ENGINEERED TRUSS MANUFACTURER'S COMPONENTS AT THIS TIME AND RESERVES THE RIGHT TO MAKE ANY CHANGES AFTER SUCH INFORMATION HAS BEEN PROVIDED FOR REVIEW. CONTRACTOR, AS PROJECT COORDINATOR, SHALL BE RESPONSIBLE FOR INSURING INFORMATION REQUESTED ABOVE HAS BEEN SUBMITTED TO ENGINEER OF RECORD IN A TIMELY MANNER WHEN AVAILABLE.

ALL PRE-ENGINEERED TRUSSES TO BE DESIGNED USING THE MOST RECENT TPI CRITERIA. TRUSSES TO BE HANDLED AND INSTALLED USING MOST RECENT BCSI RECOMMENDATIONS. TEMPORARY AND PERMANENT BRACING SHALL BE PER MOST RECENT BCSI RECOMMENDATIONS UNLESS NOTED OTHERWISE, OR MORE STRINGENT CODE REQUIREMENTS APPLY. TRUSS ENGINEER IS RESPONSIBLE FOR INDICATING ALL TRUSS TO TRUSS CONNECTORS. ALL COMPONENTS TO BE DESIGNED FOR BOTH GRAVITY AND UPLIFT LOAD CASES, INCLUDING BEAM COMPONENTS. UPON REVIEW, ENGINEER OF RECORD WILL PROVIDE A REVIEW LETTER INDICATING ANY CHANGE IN STRAPPING OR SUPPORT BASED ON THAT REVIEW. CONSTRUCTION COMMENCING PRIOR TO ENGINEER'S REVIEW IS SUBJECT TO MODIFICATION BASED ON REVIEW LETTER.

CODE COMPLIANCE NOTES:

PRE-CAST AND PRE-STRESSED CONCRETE COMPONENTS SHALL BE USED AND INSTALLED PER MANUFACTURER'S SPECIFICATIONS. PRE-CAST LINTELS HAVE BEEN REVIEWED AND PLACED BASED ON DESIGN ALLOWABLE LOAD INFORMATION PROVIDED BY CAST CRETE. THEREFORE, CAST CRETE IS A DELEGATED ENGINEER FOR THIS PROJECT. ENGINEER OF RECORD MUST APPROVE IN WRITING ANY CHANGE IN LINTEL MANUFACTURER. ALL OTHER STRUCTURAL PRE-CAST COMPONENT MANUFACTURERS MUST SUBMIT DESIGN LOAD INFORMATION TO ENGINEER OF RECORD FOR APPROVAL. ENGINEER OF RECORD RESERVES THE RIGHT TO MAKE ANY CHANGES AFTER SUCH INFORMATION HAS BEEN PROVIDED FOR REVIEW. CONTRACTOR, AS PROJECT COORDINATOR, SHALL BE RESPONSIBLE FOR INSURING INFORMATION REQUESTED ABOVE HAS BEEN SUBMITTED TO ENGINEER OF RECORD IN A TIMELY MANNER WHEN AVAILABLE.

ALL MANUFACTURED PRODUCTS SUCH AS ROOFING, WINDOWS, DOORS, ETC. ARE SHOWN HEREIN FOR ILLUSTRATION PURPOSES ONLY. THE INFORMATION SHOWN IS THE RESPONSIBILITY OF THE MANUFACTURER. THE MANUFACTURER IS RESPONSIBLE FOR THE VALIDITY OF THE COMPONENTS PROVIDED. ATTACHMENT INFORMATION GIVEN BY THE MANUFACTURER IS PROVIDED HEREIN. CONTRACTOR, AS PROJECT COORDINATOR, SHALL BE RESPONSIBLE FOR INSURING THAT THE APPROPRIATE PRODUCT/COMPONENT IS USED AND THAT

IT HAS BEEN INSTALLED PER MANUFACTURER'S SPECIFICATIONS SUCH THAT IT WILL WITHSTAND THE COMPONENTS AND CLADDING PRESSURES REQUIRED BY THE SEALED PLANS.

ENGINEER OF RECORD HAS NOT REVIEWED THIS INFORMATION FOR APPLICABILITY OR AS A FORM OF PRODUCT APPROVAL OR ENDORSEMENT.

GENERAL NOTES:

FRAMING PLAN IS DIAGRAMMATIC IN NATURE AND IS PROVIDED FOR ILLUSTRATION PURPOSES ONLY. TRUSS MANUFACTURER TO PROVIDE SEPARATE LAYOUT AND TRUSS COMPONENT DESIGN SIGNED AND SEALED BY A FLORIDA REGISTERED PROFESSIONAL ENGINEER.

ALL PRE-ENGINEERED WOOD PRODUCTS SHALL BE VERIFIED BY TRUSS MANUFACTURER. TRUSS MANUFACTURER SHALL HAVE THE AUTHORITY TO MAKE SUBSTITUTIONS FOR PRODUCTS SPECIFIED ON THE PLANS DUE TO AVAILABILITY OR ECONOMICS. CHANGES SPECIFIED BY THE TRUSS MANUFACTURER SHALL CONTROL. CHANGES MADE AFTER TRUSS ENGINEERING HAS BEEN PROVIDED TO ENGINEER OF RECORD, MUST BE APPROVED BY THE ENGINEER OF RECORD.

1. ALL EXTERIOR WALLS ARE SHEAR WALLS. NO INTERIOR SHEAR WALLS ARE REQUIRED ON THIS PROJECT.
2. ALL EXTERIOR WINDOWS & GLASS DOORS ARE REQUIRED TO BE TESTED IN ACCORDANCE WITH ANS/AMMA/NWDA 102/IS2 STANDARD AND BEAR AN AMMA OR WDMA LABEL IDENTIFYING THE MANUFACTURER, PERFORMANCE CHARACTERISTICS AND APPROVED PRODUCT TESTING ENTITY.

ALL PLUMBING, ELECTRICAL, AND MECHANICAL ROUGH INS MUST BE COMPLETE, INSPECTED, AND APPROVED BEFORE REQUESTING THE FRAMING INSPECTION.

2. DESIGN CRITERIA:

- 2.1 ROOF LOADING
 LIVE 20 PSF @ 1.25 DURATION FACTOR
 30 PSF @ 1.33 DURATION FACTOR
 DEAD 17 PSF FOR SHINGLE
 25 PSF FOR TILE
- 2.2 FLOOR LOADING
 LIVE 40 PSF @ 1.00 DURATION FACTOR
 DEAD 15 PSF
- 2.3 BALCONY LOADING
 LIVE 60 PSF @ < 100 SF. - OVER 100 SF. = 100 PSF
 DEAD 15 PSF
- 2.4 FOR FLOORING MATERIALS HEAVIER THAN 5 PSF, CONTACT ENGINEER OF RECORD FOR RECOMMENDATIONS.
- 2.5 WIND LOADING
 SEE TABLE FOR CRITERIA
 DEAD MAXIMUM 10 PSF FOR SHINGLE
 15 PSF FOR TILE

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NOTES

COVER

1. CODES AND REFERENCES:

- 1.1 FLORIDA BUILDING CODE, RESIDENTIAL 2010.
- 1.2 AMERICAN CONCRETE INSTITUTE OF STRUCTURAL CONCRETE (ACI 318)
- 1.3 AMERICAN CONCRETE INSTITUTE OF MASONRY STRUCTURES (ACI-530/ASCE-5/TMS 402 AND SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530 1/ASCE 6/TMS 602)
- 1.4 AMERICAN SOCIETY OF CIVIL ENGINEERS MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES (ASCE-7)
- 1.5 SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS LATEST EDITION
- 1.6 DESIGN SPECIFICATION FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES BY THE TRUSS PLATE INSTITUTE (TPI) LATEST EDITION
- 1.7 NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS) LATEST EDITION
- 1.8. AMERICAN PLYWOOD ASSOCIATION DESIGN / CONSTRUCTION GUIDE. (APA) LATEST EDITION

3. SOIL:

- 3.1 DESIGN ASSUMES MINIMUM ALLOWABLE SOIL PRESSURES 2,000 PSF U.N.O.
- 3.2 CONTRACTOR TO FIELD VERIFY

4. CONCRETE:

- 4.1 OPERATION INSTALLATION AND PRECEDURE TO COMPLY WITH ACI STANDARDS
- 4.2 CONCRETE & MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI AT 28 DAYS U.N.O.
- 4.3 REINFORCEMENT REBARS ASTM A615 GRADE 40 U.N.O.
- 4.4 WELD WIRE FABRIC (WWF ASTM A185) OR FIBERMESH
- 4.5 LAP SPLICES AND HOOKS SEE TABLE.

5. MASONRY:

- 5.1 MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF, "SPECIFICATION FOR MASONRY STRUCTURES (ACI 530.1/ASCE 6/TMS 602)." CONCRETE INSTITUTE, FARMINGTON HILLS, MICHIGAN; AND THE AMERICAN SOCIETY OF CIVIL ENGINEERS, RESTON, VIRGINIA; EXCEPT AS MODIFIED BY THE REQUIREMENTS OF THESE CONTRACT DOCUMENTS.
- 5.2 GENERAL SPECIFICATION FOR MASONRY STRUCTURES
- 5.2.1 TESTING OF FIELD MATERIALS FOR QUALITY CONTROL IS NOT REQUIRED BY ENGINEER FOR THIS PROJECT
- 5.2.2 COMPRESSIVE STRENGTH REQUIREMENTS IS $f_m=1500$ PSI
- 5.2.3 DETERMINATION OF COMPRESSIVE STRENGTH IS THE ALLOWABLE STRESS METHOD
- 5.2.4 UNIT STRENGTH METHOD IS NOT APPLICABLE.
- 5.2.5 QUALITY ASSURANCE IS NOT APPLICABLE.
- 5.3 PRODUCTS
- 5.3.1 MORTAR MATERIALS SHALL BE TYPE M OR S GRAY MORTAR.
- 5.3.2 MASONRY UNIT MATERIALS SHALL BE 1900 PSI MIN. CONCRETE MASONRY UNIT.
- 5.3.3 REINFORCEMENT, PRE-STRESSED TENDONS, AND METAL ACCESSORIES SHALL BE 40 KSI REBAR (MIN.)
- 5.3.4 WELDED WIRE FABRIC TO BE INSTALLED AS SPECIFIED ON PLAN SET.
- 5.3.5 STAINLESS STEEL IS NOT APPLICABLE.
- 5.3.6 COATING FOR CORROSION PROTECTION IS NOT APPLICABLE.
- 5.3.7 CORROSION PROTECTION FOR TENDONS IS NOT APPLICABLE.
- 5.3.8 PRE-STRESSING ANCHORAGE, COUPLERS, AND END BLOCKS ARE NOT APPLICABLE.
- 5.3.9 JOINT FILLERS ARE NOT APPLICABLE.
- 5.3.10 LINTELS TO BE BY CAST-CRETE U.N.O.
- 5.4 EXECUTION
- 5.4.1 PIPES AND CONDUITS ARE NOT APPLICABLE.
- 5.4.2 ACCESSORIES ARE NOT APPLICABLE.
- 5.4.3 EXPANSION AND CONTROL JOINTS SHALL BE AS INDICATED IN PLAN SET.

6. WOOD FRAMING:

- 6.1 DIMENSIONED LUMBER SHALL BE DRESSED S4S, AND SHALL BEAR THE GRADE STAMP OF THE MANUFACTURER'S ASSOCIATION.
- 6.2 ALL LUMBER SHALL BE SOUND, SEASONED, AND FREE FROM WARP.
- 6.3 FRAMING WALLS AND COLUMNS
 - 6.3.1 MINIMUM OF 3 PLY STUD COLUMNS TO BE INSTALLED AT BEAM OR GIRDER TRUSS BEARING LOCATIONS, UNLESS NOTED OTHERWISE.
 - 6.3.2 S.Y.F. #1 GRADE OR BETTER FASTEN PLYS TOGETHER USING 16d COMMON NAILS 6" O.C. AS EACH MEMBER IS APPLIED (U.N.O.)
 - 6.3.3 4 PLY OR AND LARGER STUD COLUMNS SHALL BE FASTENED TOGETHER AS STATED ABOVE PLUS CS16 COIL STRAPPING WRAPPED AROUND COLUMN WITH A 4" END CAP AT 16" O.C. OR 1/2" < THRU BOLTS AT 24" O.C.

- 6.3.4 ALL EXTERIOR FRAMING LUMBER SHALL BE #1 SYP OR EQUAL
- 6.3.5 INTERIOR LOAD BEARING (IF APPLICABLE) WALLS SPACED AT 16" O.C.
- 6.3.7 INTERIOR NON-LOAD BEARING WALLS SHALL BE UTILITY GRADE OR BETTER
- 6.3.8 INSTALL BLOCKING IN ALL WALL STUDS OVER 8'-0" AT MID-HEIGHT, AND SHEATHING JOINT. BRACE GABLE END WALLS AT 4'-0" O.C. AS SHOWN IN DRAWINGS
- 6.4 ALL LUMBER IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE TREATED OR NATURAL DURABLE WOOD.
- 6.5 PRESSURE TREATED LUMBER SHALL BE IMPREGNATED WITH A CCA SALT TREATMENT IN ACCORDANCE WITH F.S. 11-W-571 AND BARE
- 6.6 THE AMERICAN WOOD PRESERVES INSTITUTE EQUALITY MARK LP-2. A.P.A. RATED SHEATHING EXTERIOR GRADE. ALL ROOF SHEATHING TO BE INSTALLED WITH PLY CLIPS (MAXIMUM 24" O.C.). (SEE PLANS FOR SHEATHING THICKNESS.) FOR ALL SHEATHING ATTACHMENT, SEE TYPICAL NAILING SCHEDULE
- 6.6.1 ROOF: SHINGLE, 7/16" MIN. THICK SUPPORTED OVER 24" MAX. SPAN
- 6.6.2 TILE, 1/2" MIN. THICK SUPPORTED OVER 24" MAX. SPAN
- 6.6.3 WALL: 7/16" MIN. THICK SUPPORTED OVER 24" MAX. SPAN
- 6.6.4 FLOOR: CARPET, VINYL, WOOD, ETC., 3/4" MIN. TONGUE AND GROOVE SUPPORTED OVER 24" MAX. SPAN
- 6.6.5 CERAMIC TILE, MARBLE, ETC., SEE MANUFACTURERS RECOMMENDATIONS.
- 6.7 ALL NAILING AND BOLTING SHALL COMPLY WITH AMERICAN INSTITUTE OF TIMBER CONSTRUCTION REQUIREMENTS. ALL NAILS EXPOSED TO THE EXTERIOR SHALL BE GALVANIZED.
- 6.8 ALL CONNECTION HARDWARE SHALL BE GALVANIZED AND SUPPLIED BY SIMPSON STRONG TIE CO., USP, OR EQUIVALENT. SUBMIT CUT SHEETS FOR ALL CONNECTION HARDWARE TO ENGINEER FOR APPROVAL. ALL NAIL HOLES SHALL BE FILLED OR AS PRESCRIBED BY THE MANUFACTURER.
- 6.9 BRACING: TEMPORARY BRACING OF THE ROOF SYSTEM SHALL BE INSTALLED PER HIB-91 RECOMMENDATIONS AND SHALL BE UTILIZED AS THE PERMANENT BRACING FOR THE ROOF SYSTEM (U.N.O.)
- 6.10 ALL WOOD FRAMING SHALL BE IN COMPLIANCE WITH THE LATEST NDS EDITION FOR WOOD CONSTRUCTION.
- 6.11 ALL WOOD LOAD BEARING COMPONENTS TO BE #1 SYP. NON LOAD BEARING COMPONENTS MAY BE SPF (UTILITY GRADE OR BETTER)

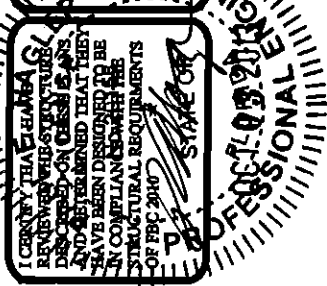
NOTES:

MISSING DOWELS: WHERE FOOTING DOWELS ARE PLACED INCORRECTLY OR MISTAKENLY ELIMINATED, REPLACE DOWEL AT PROPER LOCATION W/ GRADE 40 #5 BAR. INSTALL IN SLAB W/ 5" MINIMUM EMBEDMENT, USE EPOXY GROUT.

MISSING TRUSS ANCHORS: TRUSSES WHICH ARE PLACED SUCH THAT AN EMBEDMENT ANCHOR IS MISPLACED OR MISSING MAY BE FASTENED TO THE MASONRY BOND BEAM USING ONE SIMPSON MTSML6 W/ (4) 1/4"x2 1/4" TITEN SCREWS AND 7-10 D NAILS IN TRUSS. MAS CAP IS 840 #.

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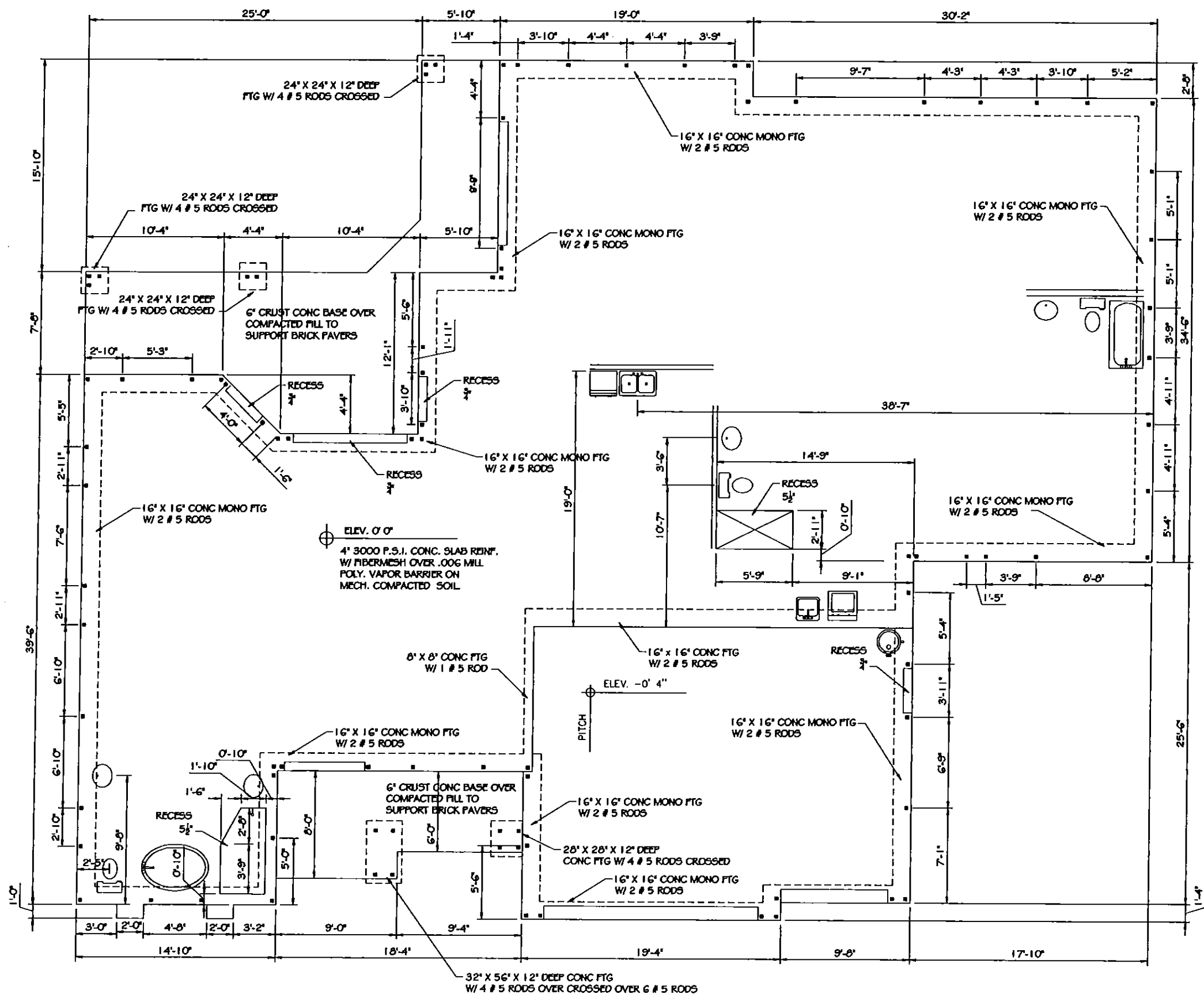
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NOTES

S1



NOTES

- 1) THE FOUNDATION SYSTEM FOR THIS PLAN IS DESIGNED ACCORDING TO RAZAK GEO TECHNICAL ENGINEERS INC. DATED 5-28-2012 ALLOWABLE BEARING PRESSURE OF 1800 P.S.F.
- 2) FOOTINGS TO BEAR MIN. 12" BELOW GRADE.
- 3) FOOTINGS TO BEAR ON SOIL PREPARED ACCORDING TO ATTACHED SOILS REPORT FILL COMPACTED TO 95% MOD. PROCTOR BETWEEN LESS THAN 12" LIFTS.
- 4) ALL BEARING SOILS TO BE FREE OF DEBRIS AND ORGANIC MATERIAL.
- 5) REFER TO STRUCTURAL ENGINEER NOTES.

SYNTHETIC FIBER REINFORCEMENT IN CONCRETE FOR SLAB-ON GRADE SHALL COMPLY WITH FBC SECT. 1911.2 (EXCEPTION 1)

DRIVEWAY SPEC:
 DRIVEWAY NOT IN RIGHT OF WAY AND ALL SIDEWALKS TO BE 4" 3000PSI CONC. W/ FIBERMESH.
 DRIVEWAY IN RIGHT OF WAY TO BE 6" 3000 PSI CONCRETE WITH FIBERMESH AND WIRE REINFORCEMENT.

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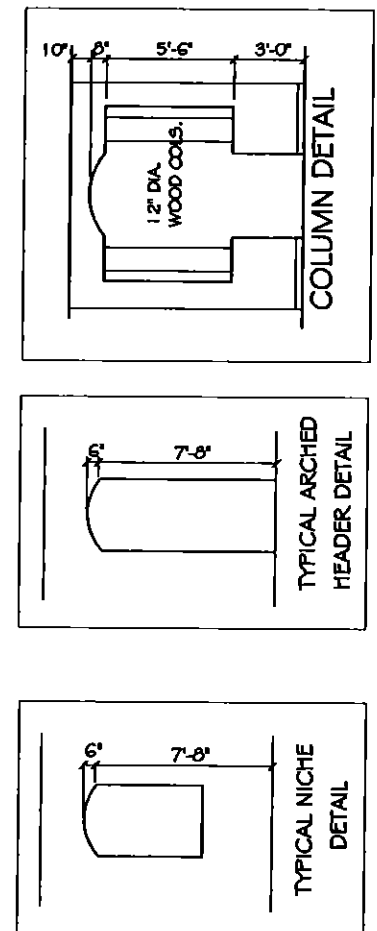
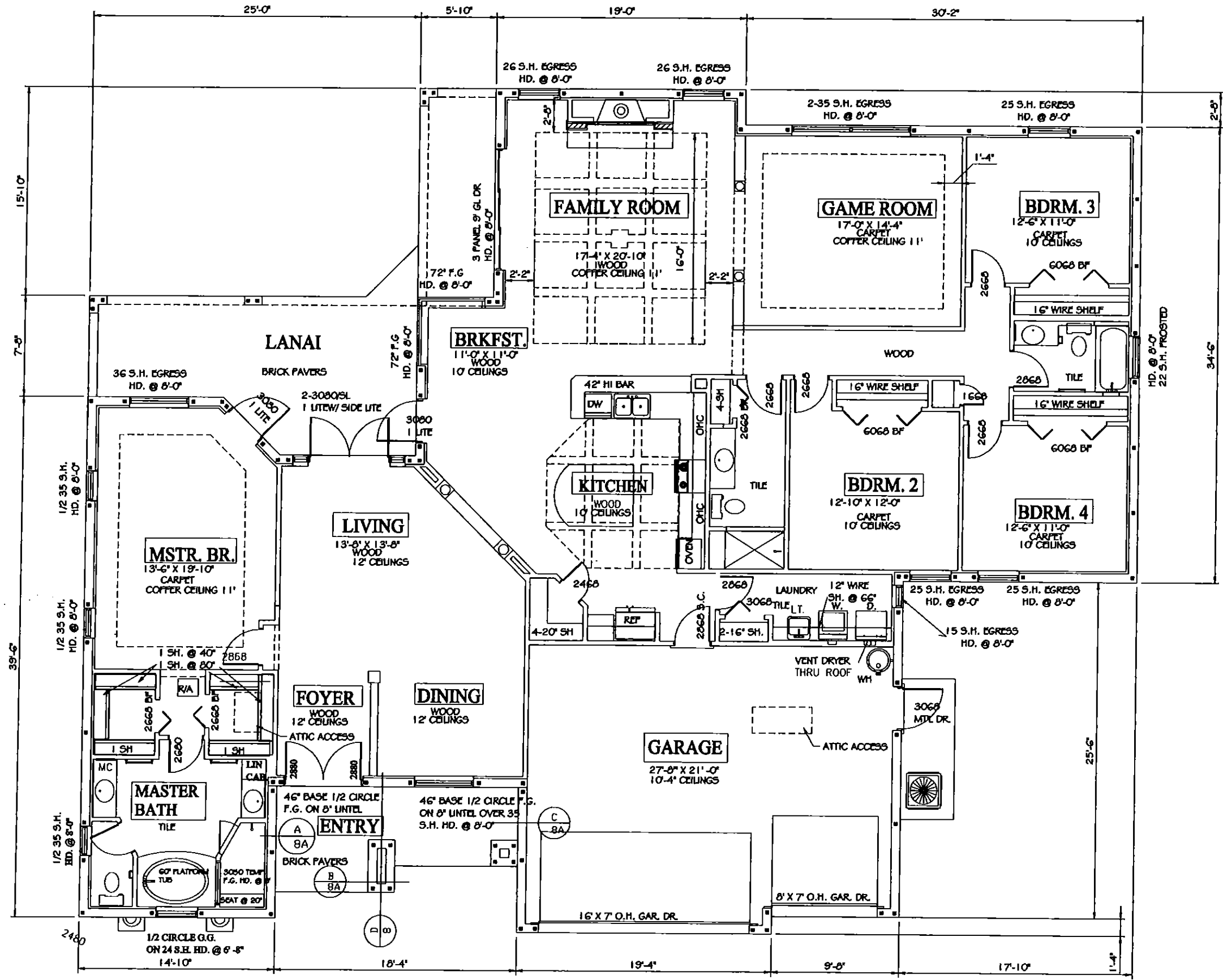
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TERMITE SPECIFICATIONS:

INSTALL "BORA-CARE" TERMITE PROTECTION SYSTEM PER MANUF. SPECIFICATIONS

FOUNDATION PLAN

1



SQUARE FOOTAGES

LIVING AREA-	2984 S.F.
GARAGE	- 614 S.F.
LANAI	- 342 S.F.
ENTRY	- 130 S.F.
TOTAL	- 4070 S.F.

FLOOR PLAN

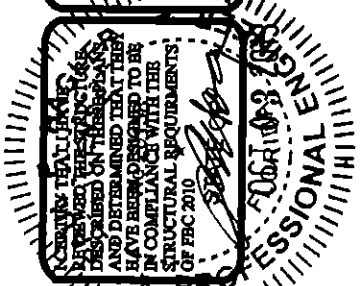
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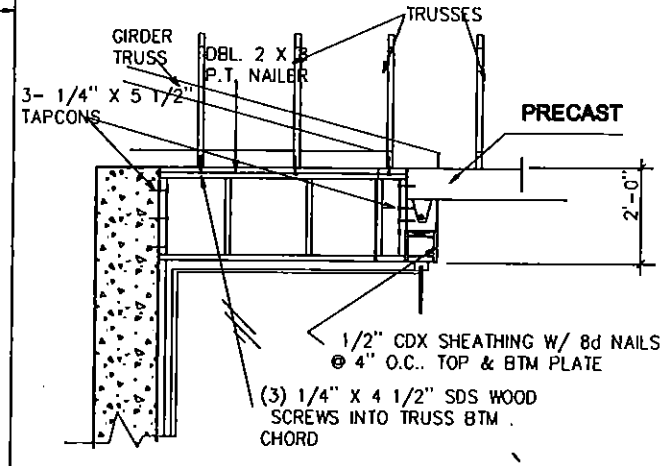
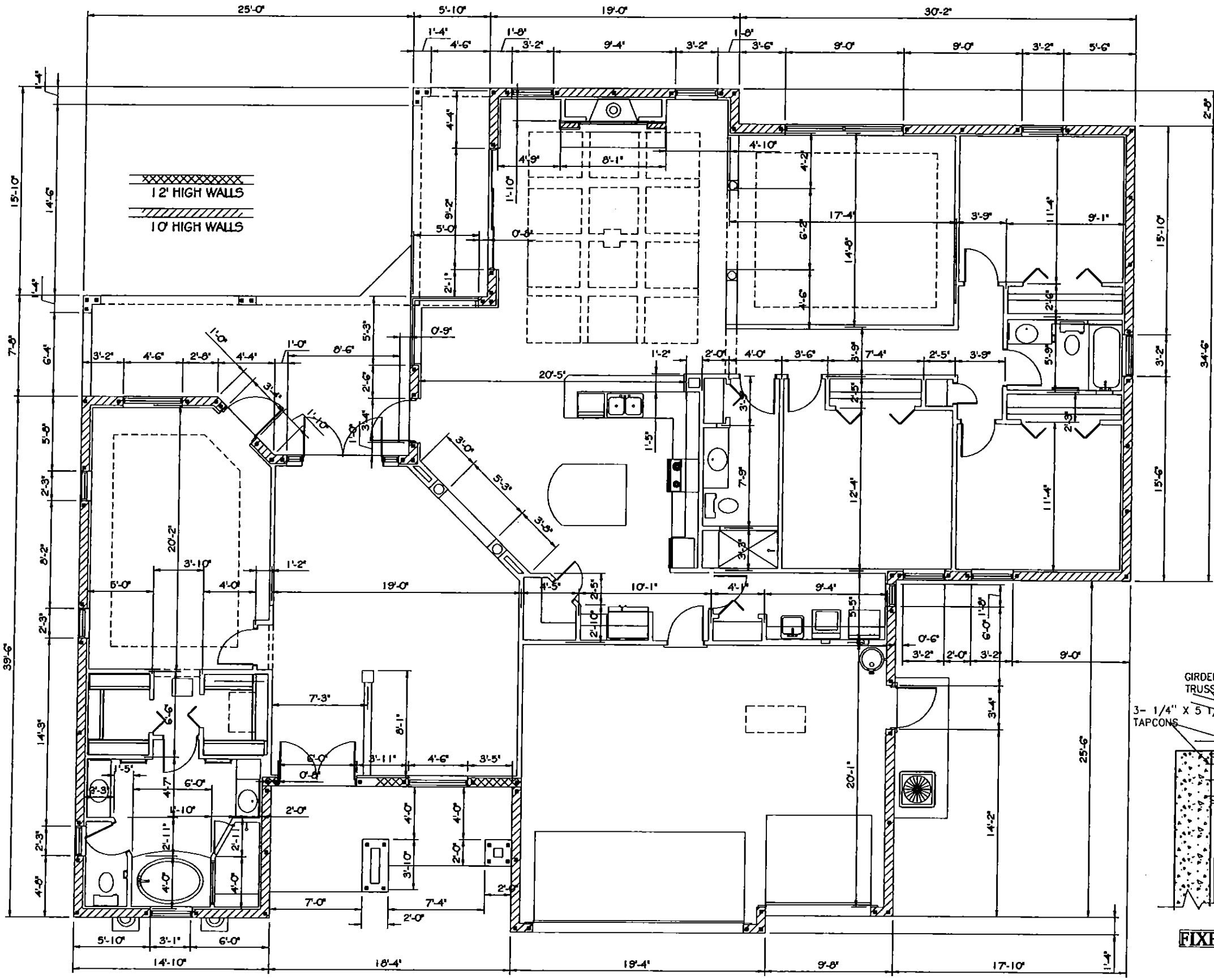
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FIXED GLASS HEADER DETAIL

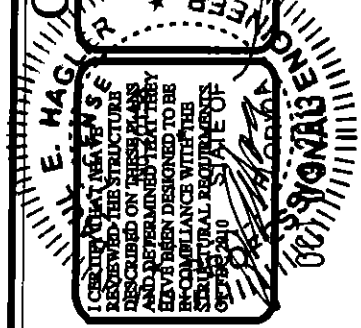
DIMENSION PLAN

3

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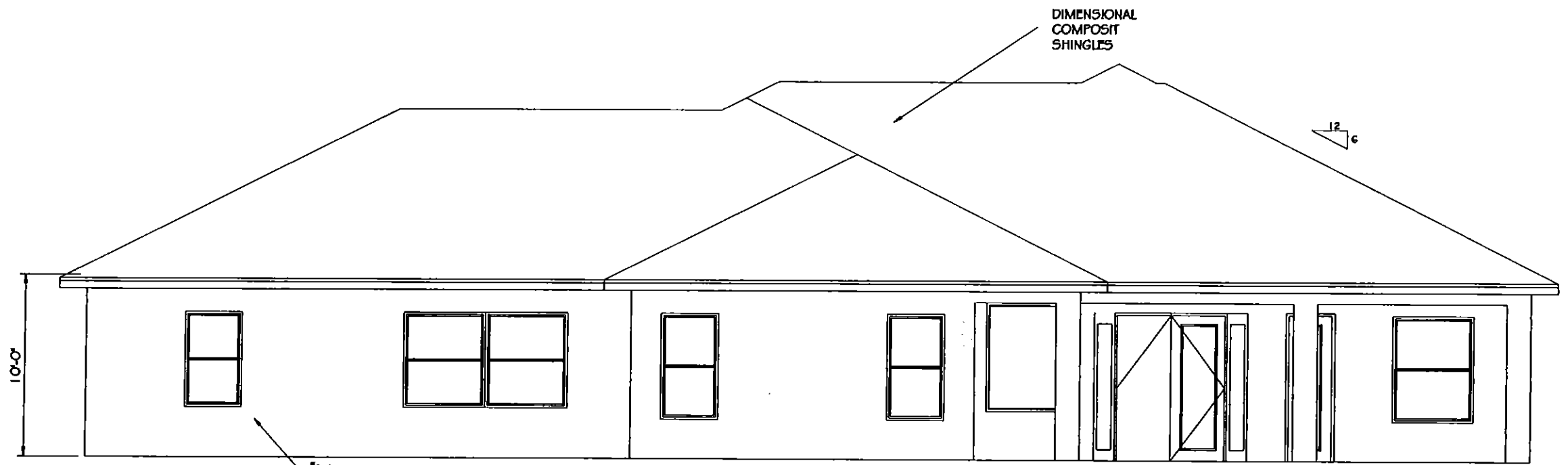
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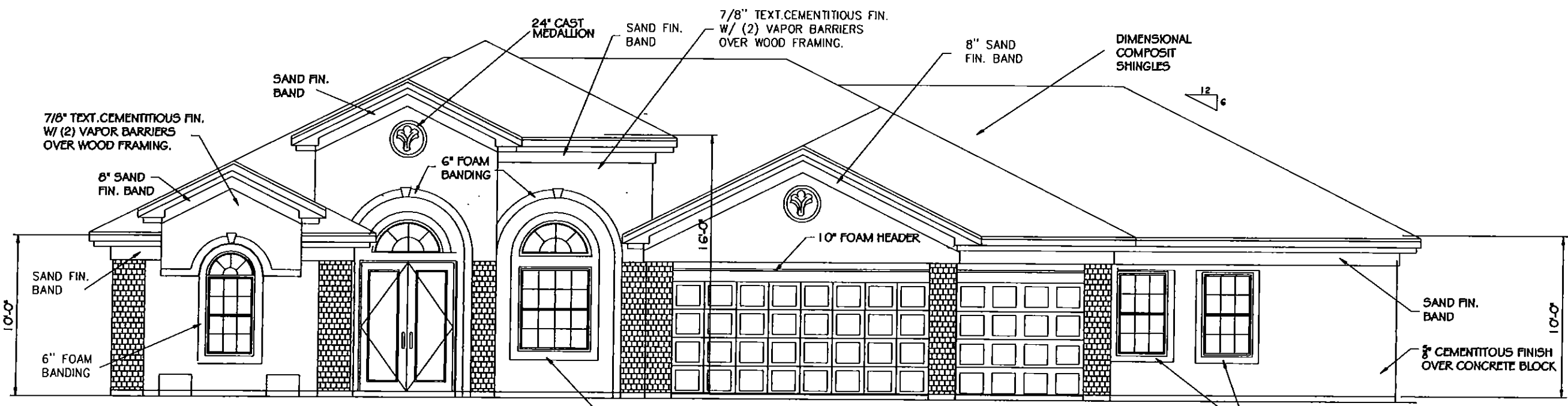
3/8" CEMENTITIOUS FINISH
OVER CONCRETE BLOCK

DIMENSIONAL
COMPOSIT
SHINGLES

12/6

10'-0"

REAR ELEVATION



7/8" TEXT. CEMENTITIOUS FIN.
W/ (2) VAPOR BARRIERS
OVER WOOD FRAMING.

SAND FIN.
BAND

24" CAST
MEDALLION

SAND FIN.
BAND

7/8" TEXT. CEMENTITIOUS FIN.
W/ (2) VAPOR BARRIERS
OVER WOOD FRAMING.

8" SAND
FIN. BAND

DIMENSIONAL
COMPOSIT
SHINGLES

12/6

10'-0"

SAND FIN.
BAND

8" SAND
FIN. BAND

6" FOAM
BANDING

16'-0"

10" FOAM HEADER

SAND FIN.
BAND

10'-0"

3/8" CEMENTITIOUS FINISH
OVER CONCRETE BLOCK

FRONT ELEVATION-B

6" FOAM
BANDING

6" FOAM
BANDING

ELEVATIONS

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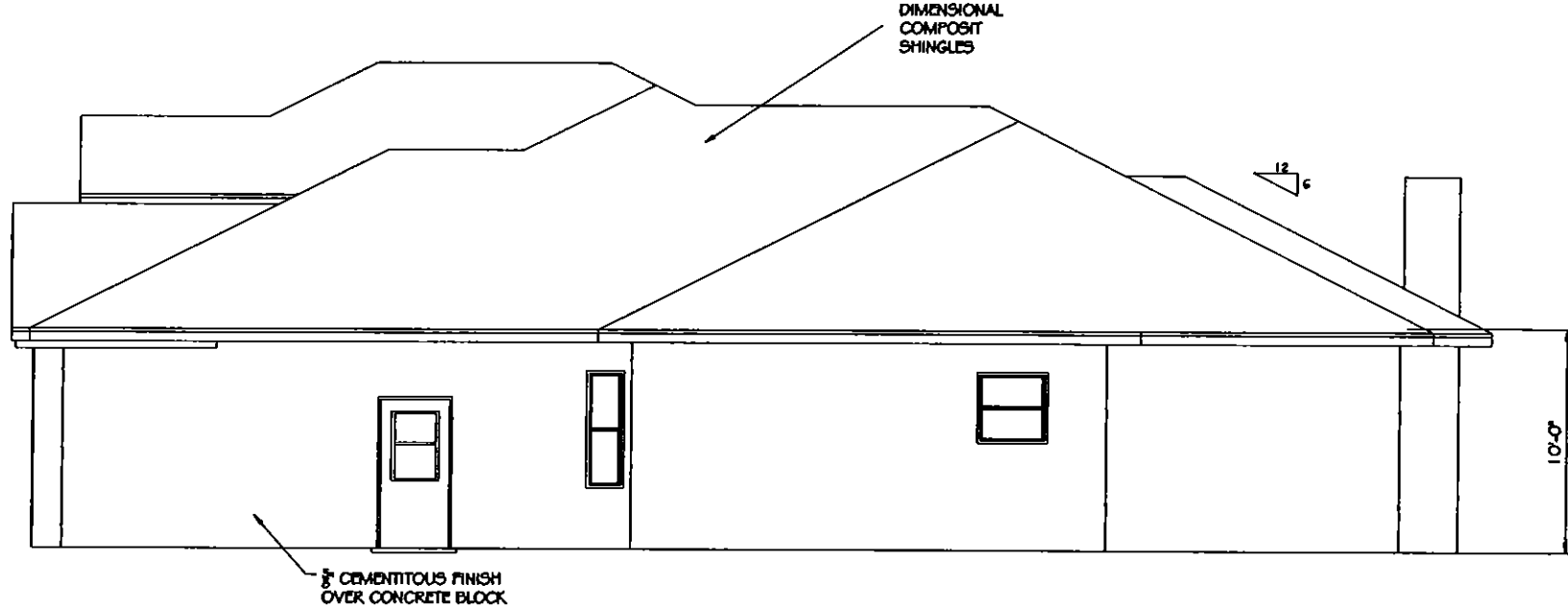


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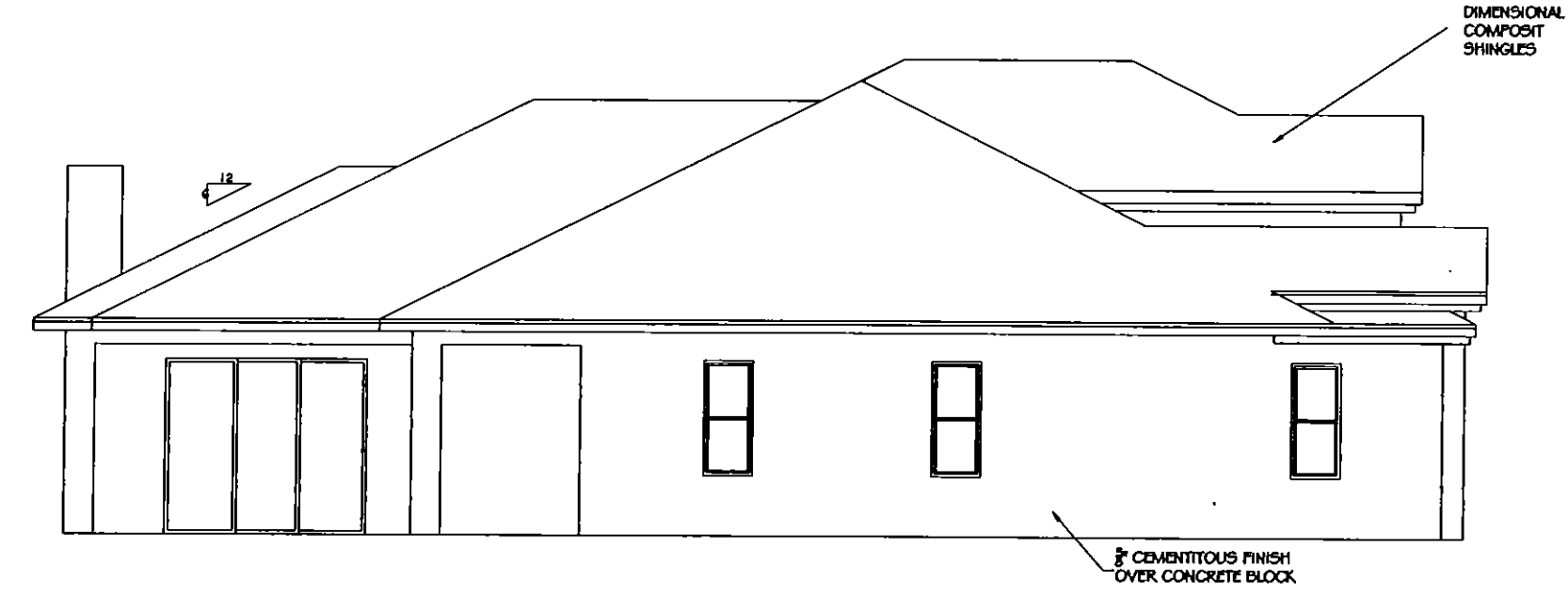
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4



LEFT SIDE ELEVATION



RIGHT SIDE ELEVATION

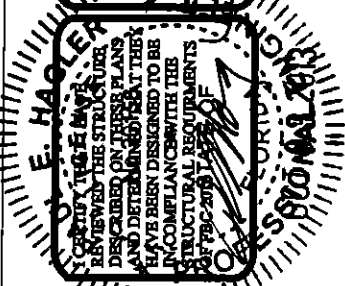
ELEVATIONS

5

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9400 RIVER CROSSING BLVD
NEW PORT RICHEY FL. 34655
727-376-6831

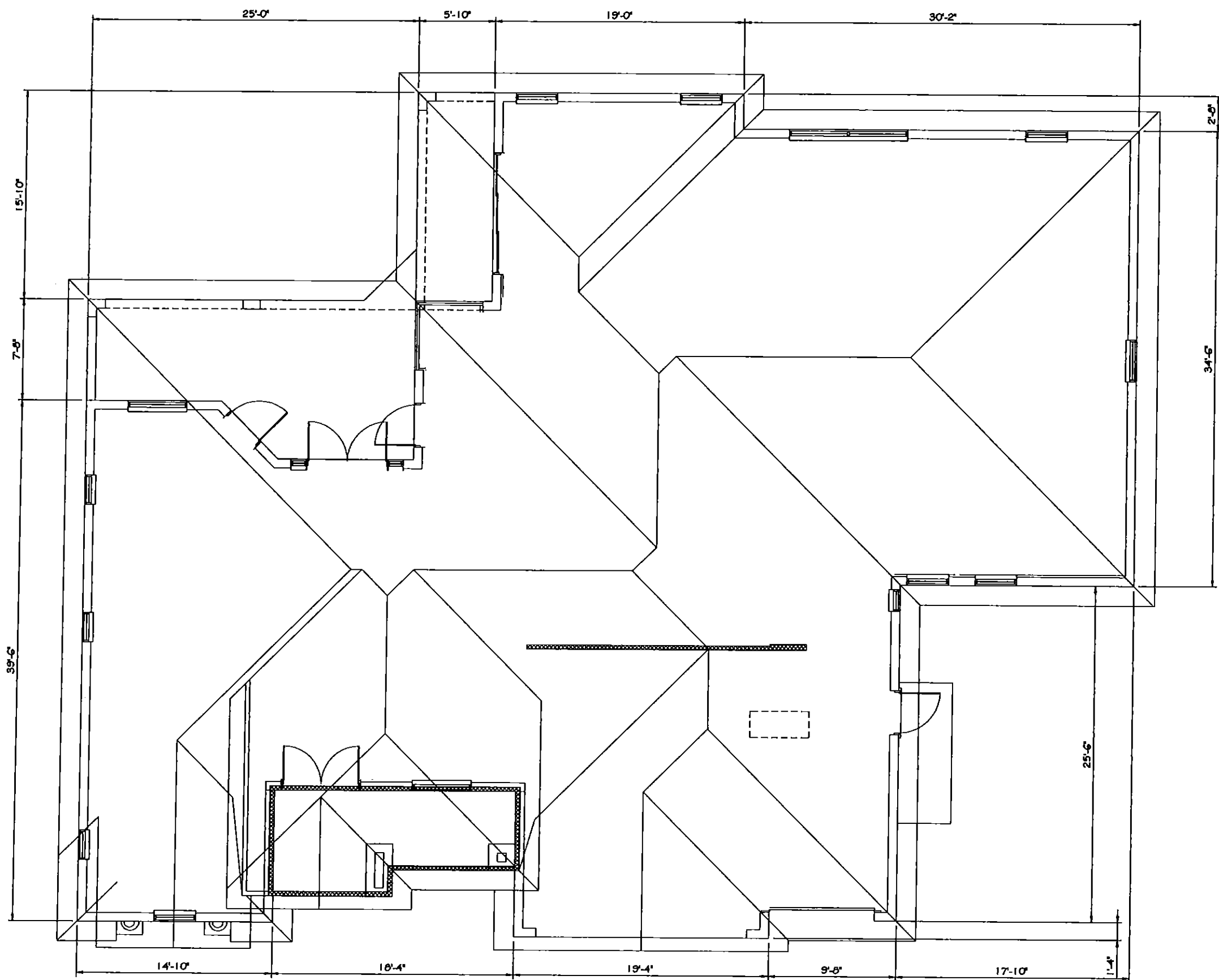
PLAN DATE

0 MAIN ST
SAFETY HARBOR, FL.



CUSTOM 2984

PAUL E HAGLER
FPE 20158
1280 HEATHER RIDGE BLVD.
DUNEDIN, FL. 34698
727-738-9025



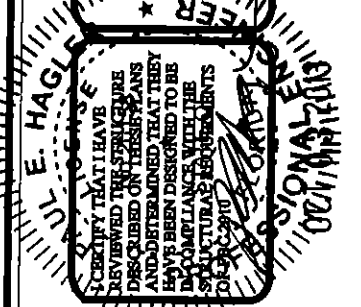
ROOF PLAN

6

**DEEB FAMILY
HOMES, LTD.**
9400 RIVER CROSSING BLVD
NEW PORT RICHEY FL. 34655
727-376-6831

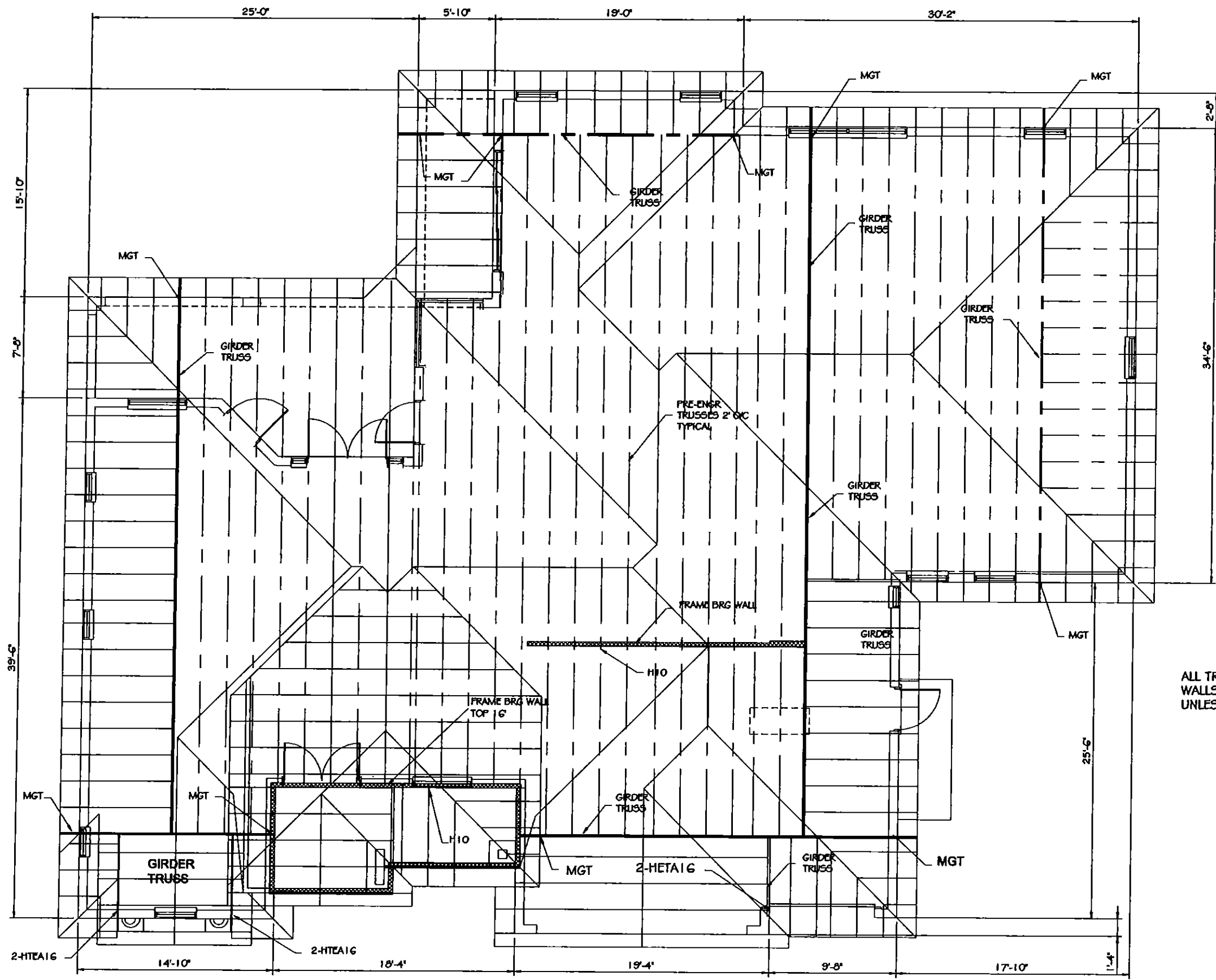
PLAN DATE

0
MAIN ST
SAFETY HARBOR, FL.



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ALL TRUSS THAT BRG ON MASONRY WALLS SHALL HAVE 1-HETA 16 CONNECTOR UNLESS NOTED ON THE PLAN

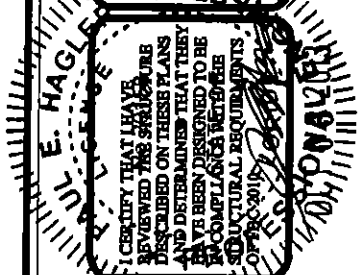
ROOF FRAMING PLAN

6A

DEEB FAMILY HOMES, LTD.
9400 RIVER CROSSING BLVD
NEW PORT RICHEY FL. 34655
727-376-6831

PLAN DATE

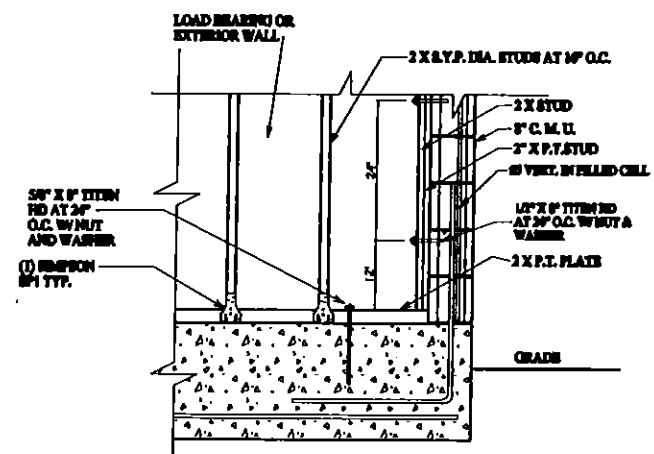
0 MAIN ST
SAFETY HARBOR, FL.



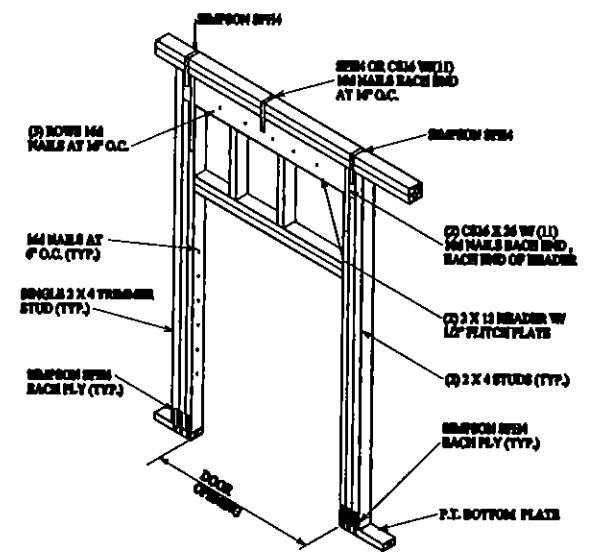
CUSTOM 2984

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FPE 20158
1280 HEATHER RIDGE BLVD.
DUNEDIN, FL. 34698
727-738-9025

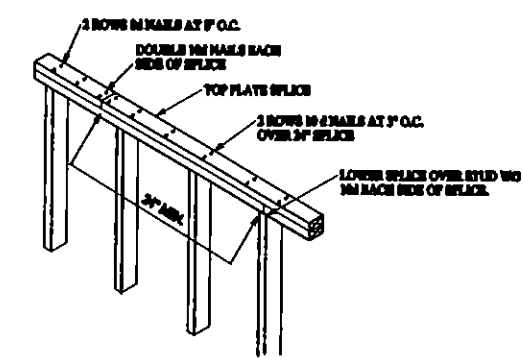
NOTE: INSTALL TYPED ANCHORS WITHIN 6" OF TOP AND BTM. PLATE AND 24" O.C. BETWEEN END ANCHORS.



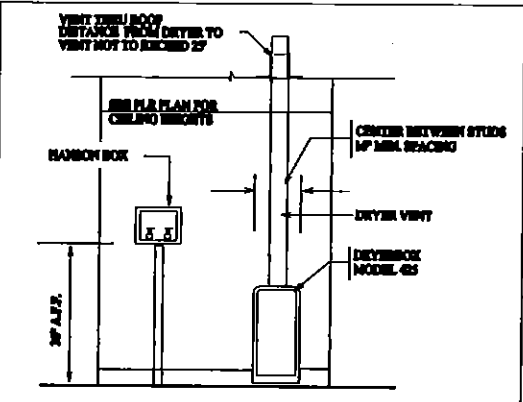
WOOD STUD CONNECTION TO MASONRY WALL



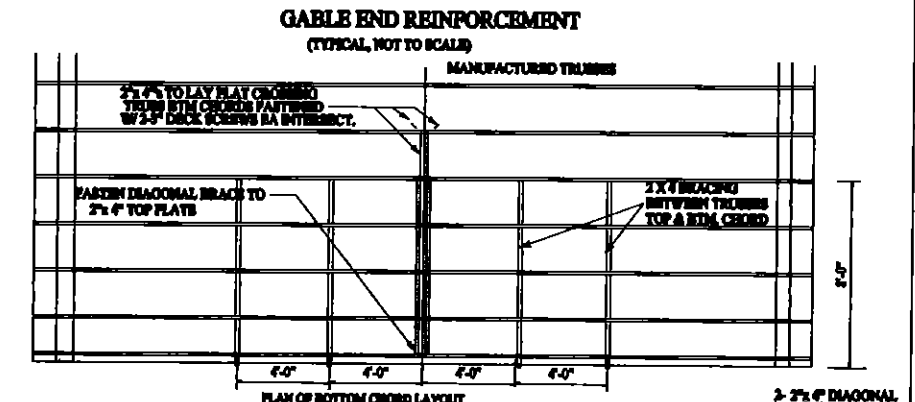
TYPICAL LOAD BEARING HEADER DETAIL



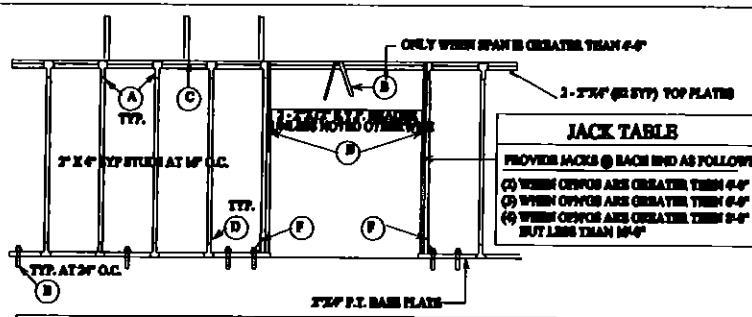
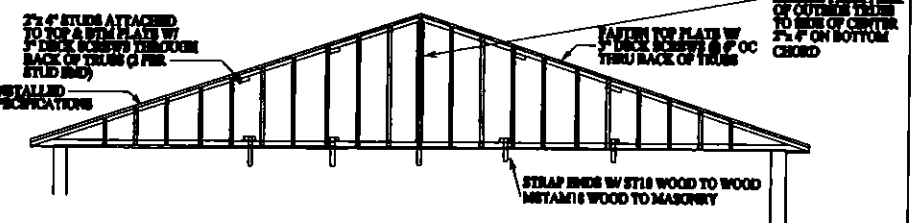
TOP PLATE SPLICE DETAIL



TYPICAL LAUNDRY PLUM. WALL



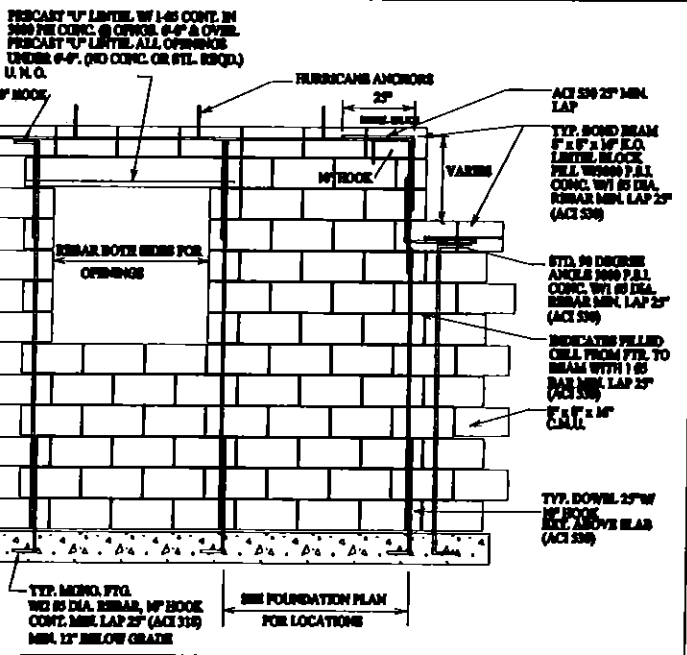
GABLE END REINFORCEMENT (TYPICAL, NOT TO SCALE)



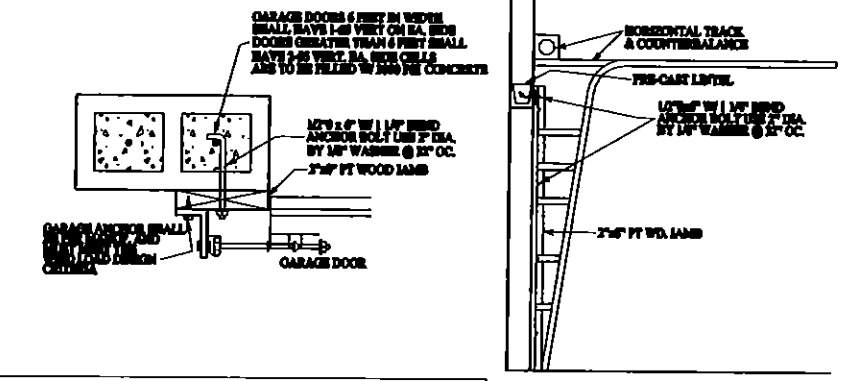
TYPICAL 1 ST. INT. BEARING WALL U.N.O.

CONNECTOR LEGEND

(A)	SIMPSON SP2 W/ 6-10# x 12
(B)	(4) SIMPSON LETAH W/ 3-10#
(C)	END FROM TRUSS TO TOP PLATE
(D)	SIMPSON SP1 W/ 6-10# x 12
(E)	50" X 8" ANCHOR BOLT W/ 3" WASHER AT 24" O.C.
(F)	SIMPSON LETAH W/ 3-10# AND 3" ROD HEAD 1/4" OF BRACKET ONLY APPLIES WHEN TRUSS IS UPLIFT AND ROOF LOAD APPLIED



BLOCK WALL/REINFORCEMENT



GARAGE DOOR CONNECTION DETAIL

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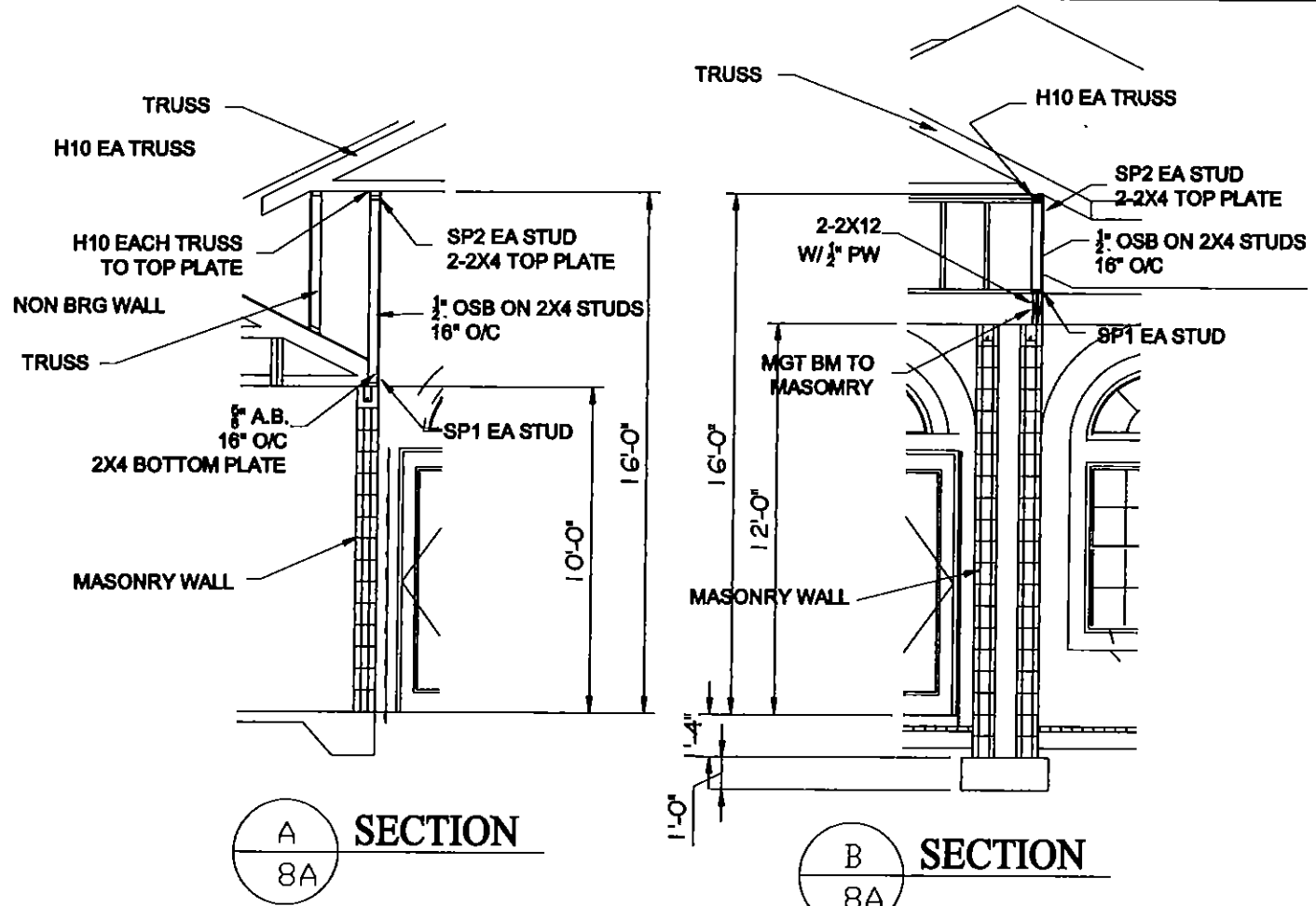
0 MAIN ST
 SAFETY HARBOR, FL.

PLAN DATE

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 HOMES, LTD.
 9400 RIVER CROSSING BLVD
 NEW PORT RICHEY FL. 34655
 727-376-6831

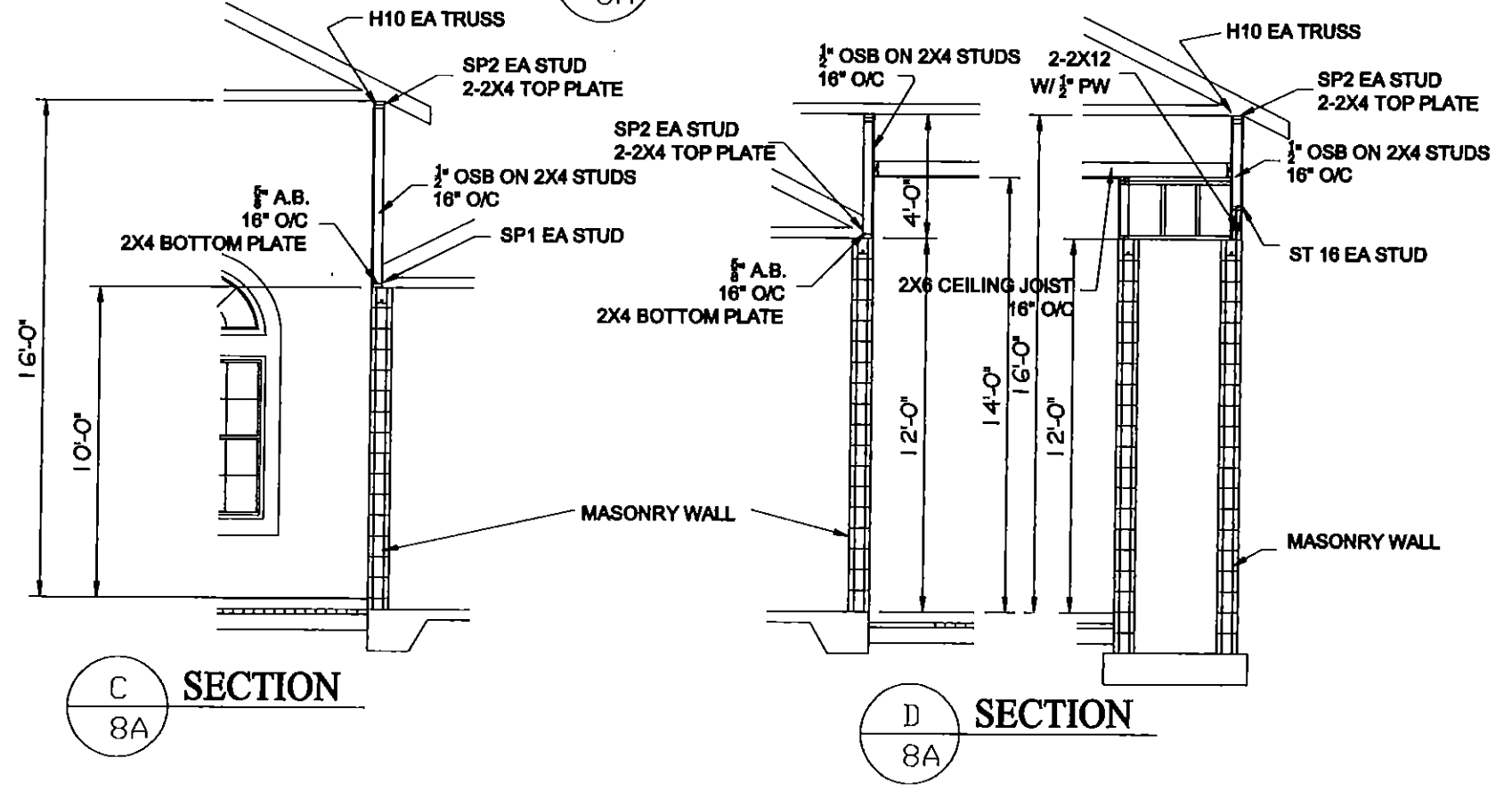
SECTIONS

8



A SECTION
8A

B SECTION
8A

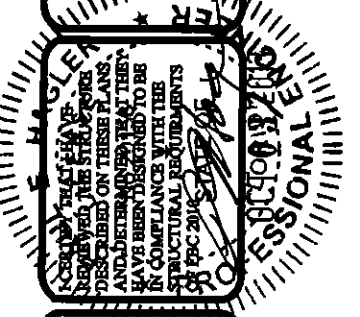


C SECTION
8A

D SECTION
8A

CUSTOM 2984

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1280 HEATHER RIDGE BLVD.
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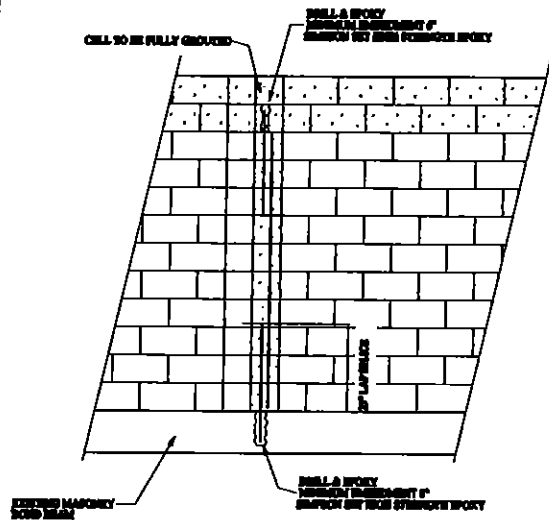
0 MAIN ST
SAFETY HARBOR, FL.

PLAN DATE

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SECTIONS

8A



TYP. RETROFIT VERT. DOWEL CONDITION

NOTE:
MISSING DOWELS: WHEN MISSING DOWELS ARE PLACED INCORRECTLY OR MISTAKENLY ELIMINATED, REPLACE DOWEL AT PROPER LOCATION W/ GRADE AS SHOWN. INSTALL IN SLAB W/ 2\"/>

MISSING ANCHOR BOLTS AT BEARING WALL:

EXTERIOR BEARING WALL:

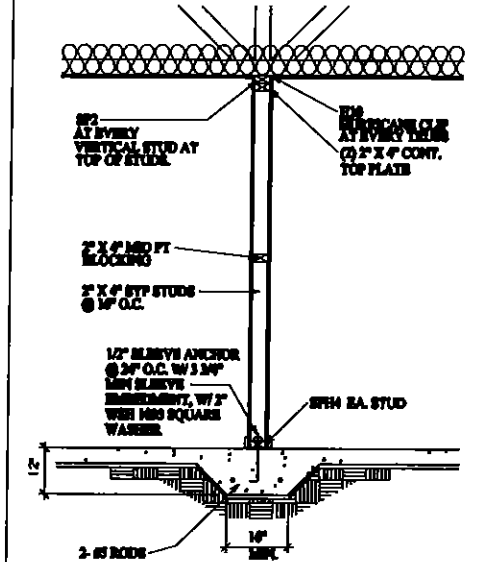
IN ADDITION TO THE ORIGINAL PLACEMENT REQUIREMENTS:

- 1) 3/4\"/>

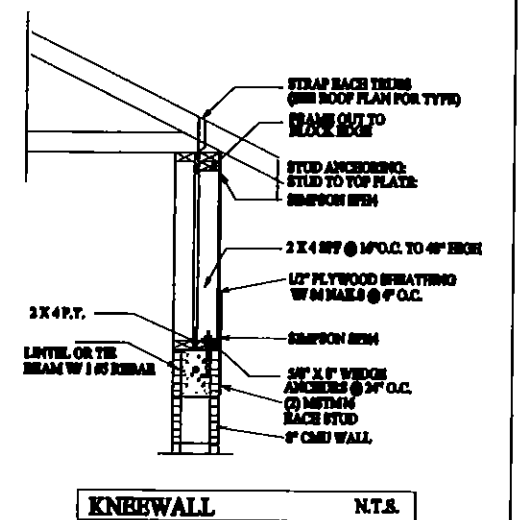
INTERIOR BEARING WALL:

IN ADDITION TO THE ORIGINAL PLACEMENT REQUIREMENTS:

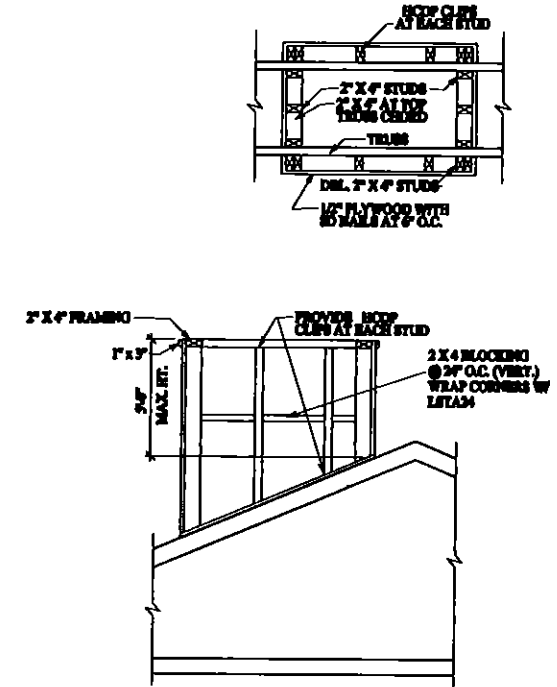
- 1) 3/4\"/>



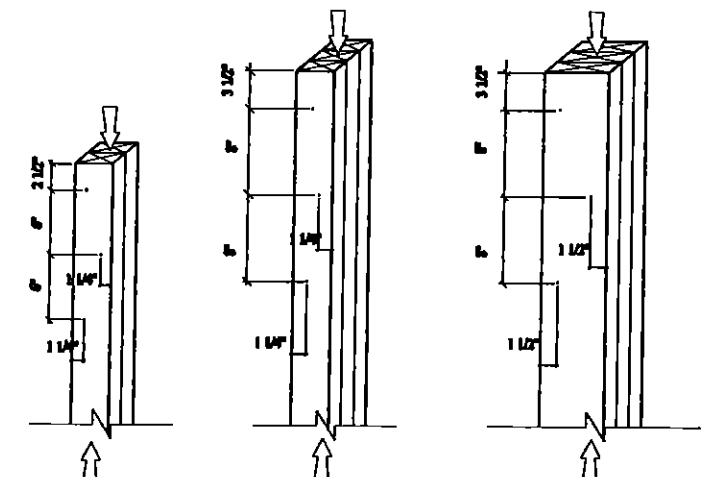
BEARING PARTITION



KNEEWALL N.T.S.



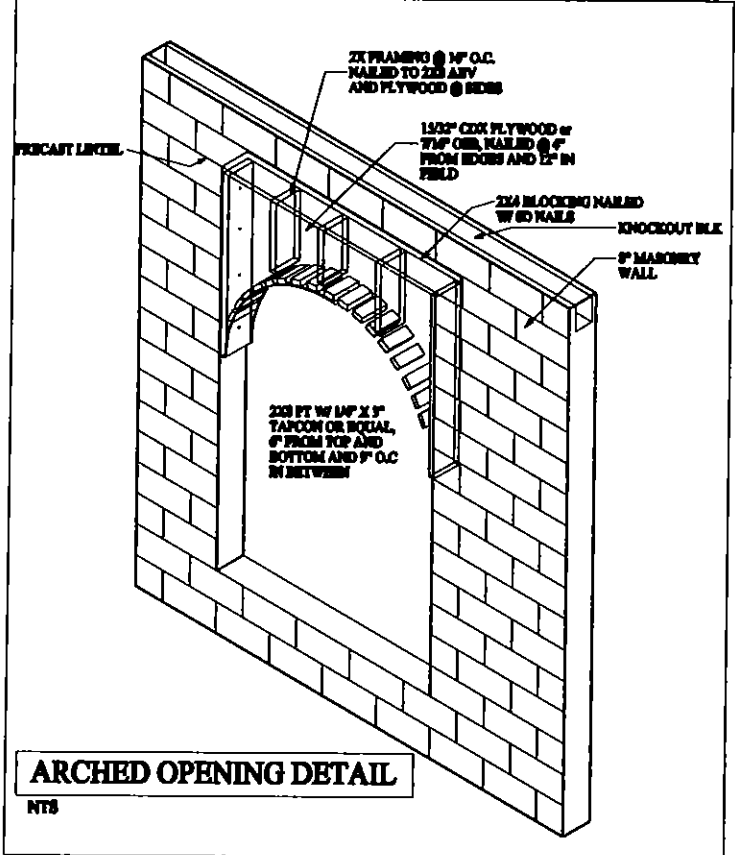
DETAIL-CHIMNEY



- (A) 2\"/>
- (B) 2\"/>
- (C) 2\"/>

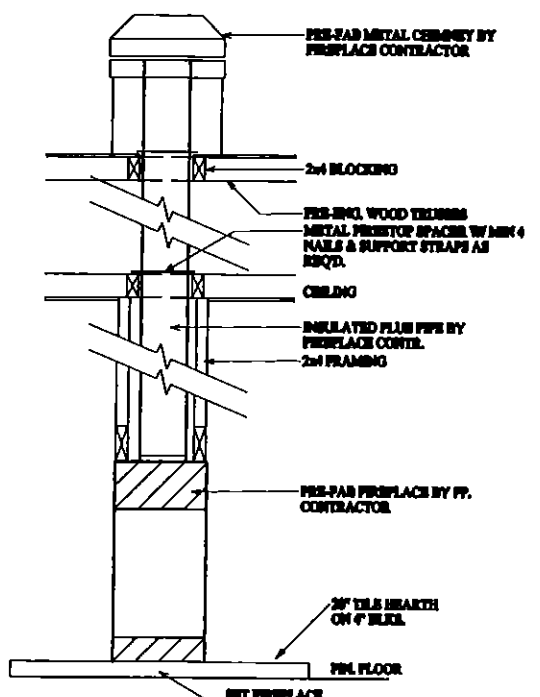
NOTE:
1) ADJACENT NAILS ARE DRIVEN FROM OPPOSITE SIDES OF THE COLUMN
2) ALL NAILS PENETRATE AT LEAST 3/4\"/>

TYP. NAILING SCHEDULE FOR BUILT-UP COLUMNS



ARCHED OPENING DETAIL

N.T.S.



SECTION - FIREPLACE

N.T.S.

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CONTRACTOR THAT HAVE REVIEWED THE STRUCTURE AND DETERMINED THAT THEY HAVE BEEN DESIGNED TO BE IN COMPLIANCE WITH THE STRUCTURAL REQUIREMENTS OF THE 2010

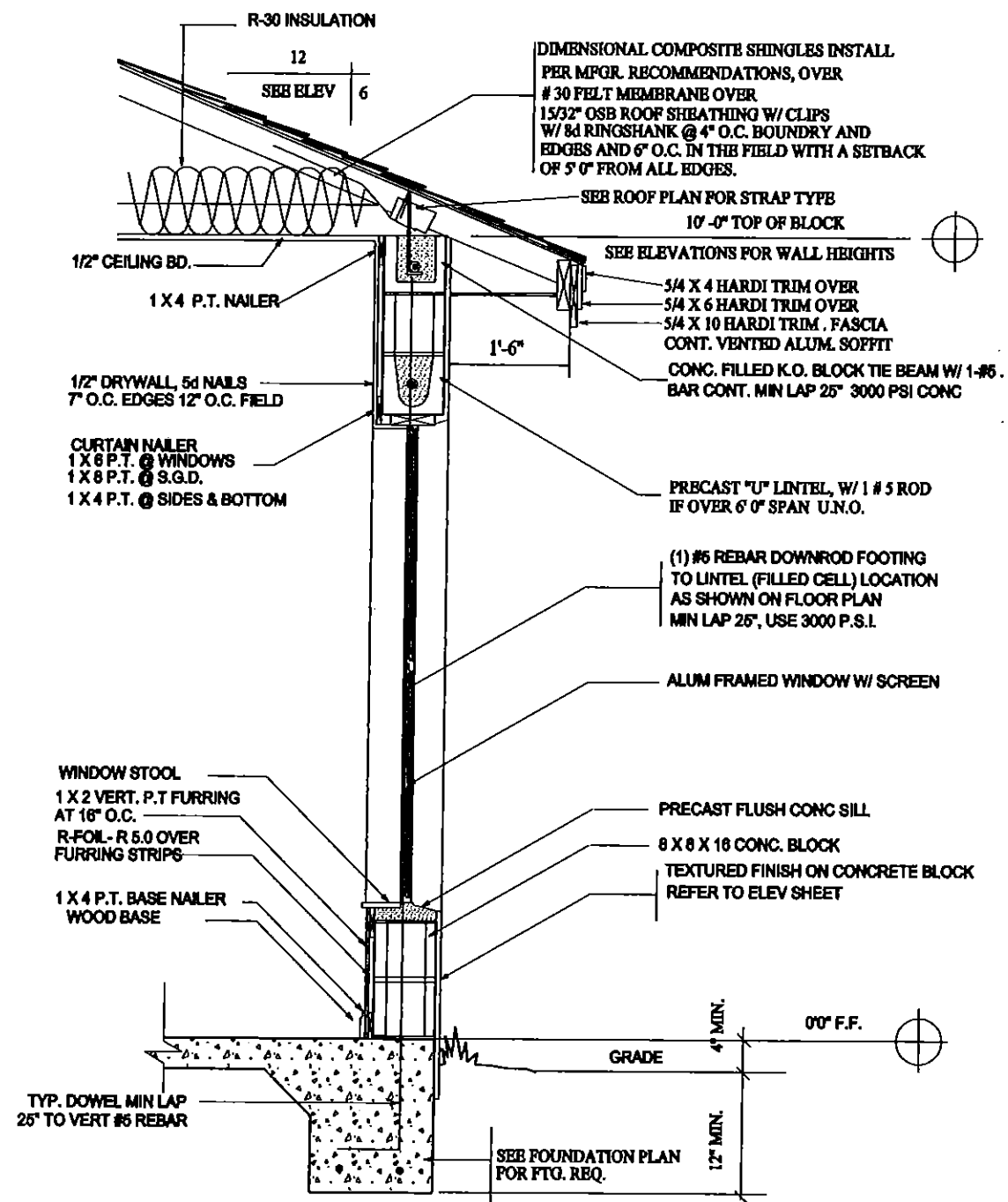
0
MAIN ST
SAFETY HARBOR, FL.

PLAN DATE

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9400 RIVER CROSSING BLVD
NEW PORT RICHEY FL. 34655
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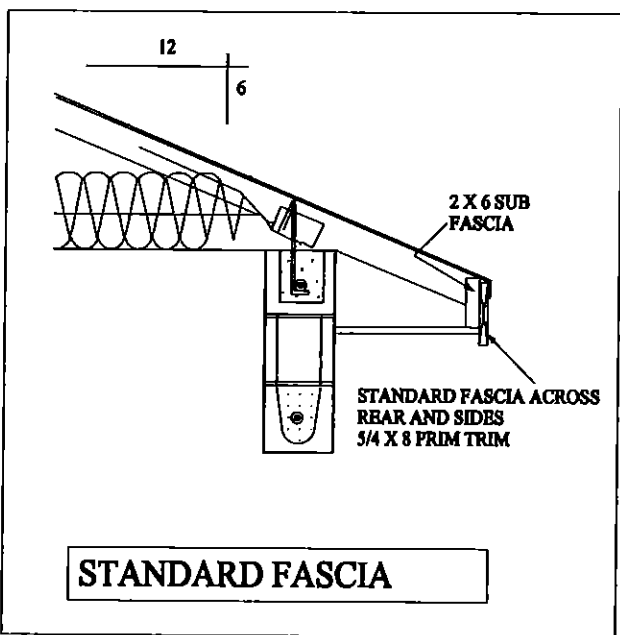
SECTIONS

9



TYPICAL WALL SECTION

TERMITE SPECIFICATIONS:
 INSTALL "BORA-CARE" TERMITE PROTECTION SYSTEM
 PER MANUF. SPECIFICATIONS



STANDARD FASCIA

CONNECTOR TABLE

SIMPSON	FLORIDA PRODUCT NUMBERS PER INDEX 9-27-09
MBH13.05/11.08	10408.12
H2	10408.10
H8	10408.18
H10	10408.8
LGT2	11470.8
MGT	11470.7
LSTA10	10882.4
LSTA24	10882.4
SP1	10408.41
SP2	10408.42
HTS20	10408.23
HTB16	10408.22
META6	11473.17
L30	10408.11
MSTANG4	11473.19
MSTANG8	11473.19
MSTANG8	11473.19
MSTC80	11473.10
CS16	10992.1
SPH	10408.40
SPB	10408.47
HT4	11408.2
HT8	11408.2
ABL08	10408.5

SECTIONS

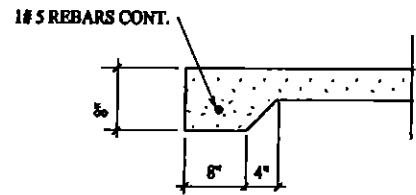
10

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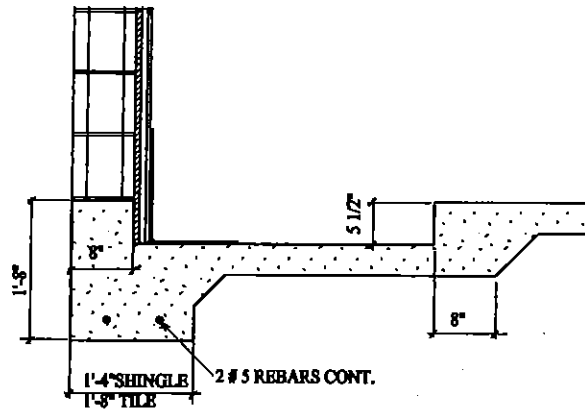
PLAN DATE

0 MAIN ST SAFETY HARBOR, FL.

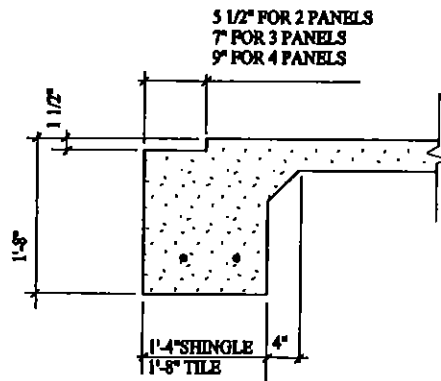
E. HAGLER CUSTOM 2984
 PAUL E HAGLER
 * FPE 20158
 1280 HEATHER RIDGE BLVD.
 LUDWIGSDALE, FL. 34698
 813-738-9025



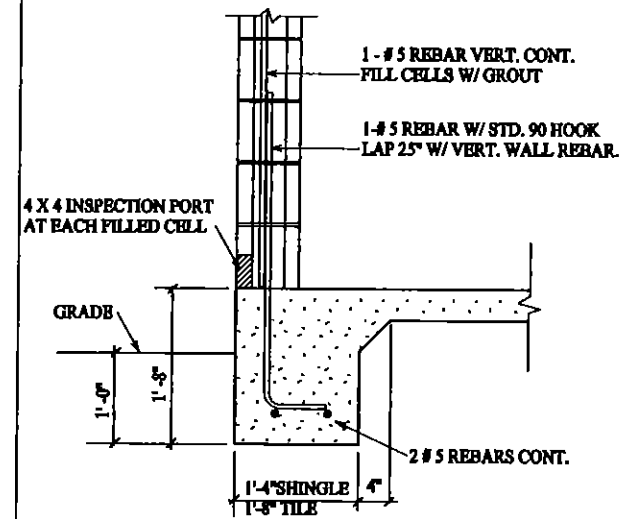
8" THICKENED SLAB (J)



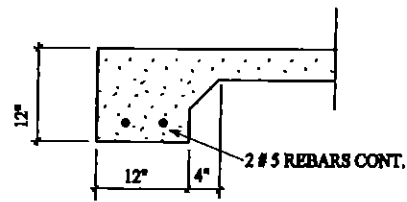
SHOWER RECESS (G)



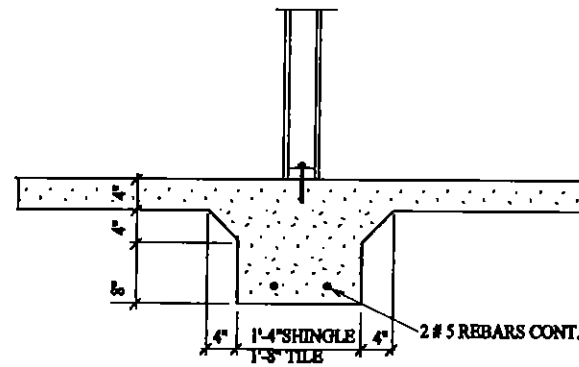
SLIDING GLASS DR. RECESS (D)



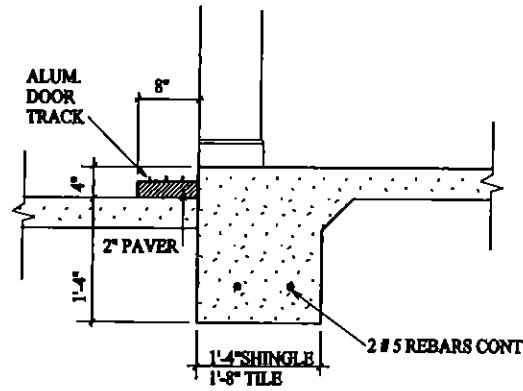
TYPICAL ONE STORY (A)



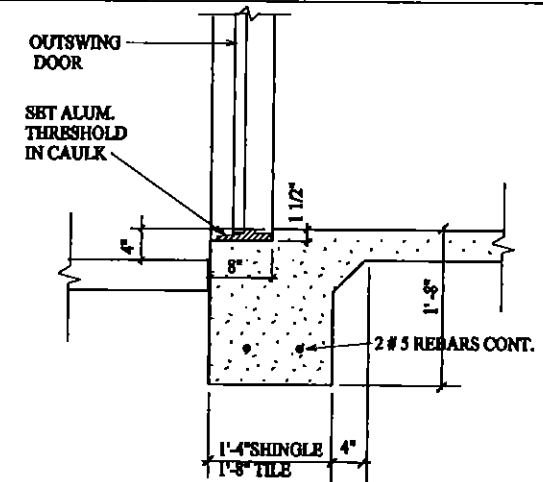
12" THICKENED SLAB (K)



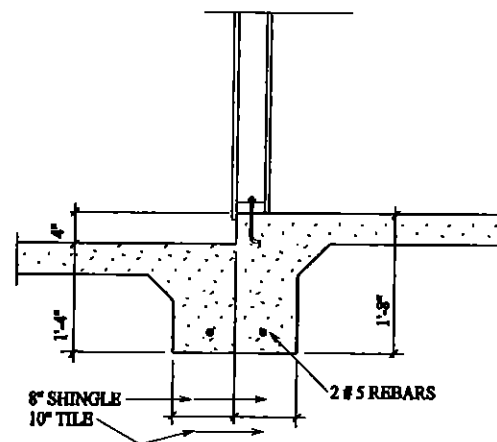
INTERIOR BEARING FTG. (H)



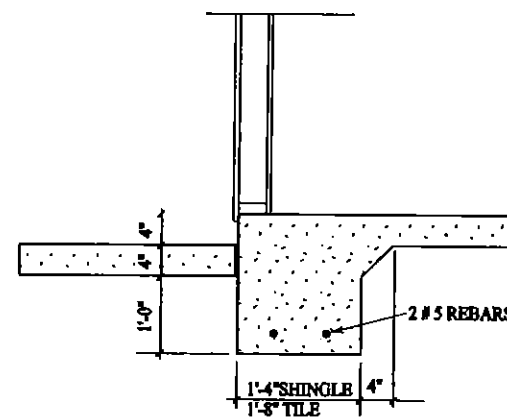
EXTERIOR POCKET S.G.D. (E)



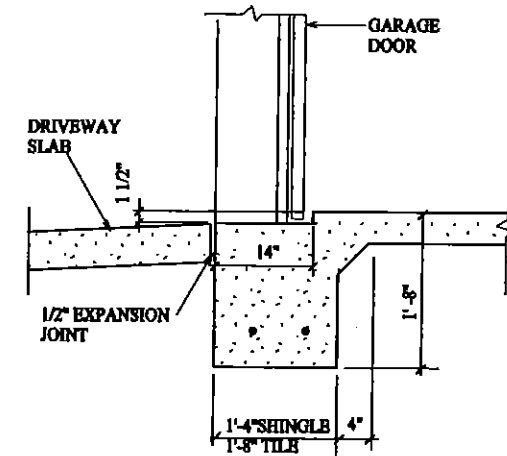
EXTERIOR DOOR RECESS (B)



BEARING GARAGE STEP (I)



NON-BRG. GARAGE STEP (F)



GARAGE DOOR RECESS (C)

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SECTIONS